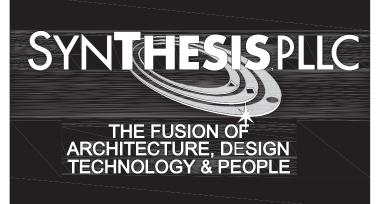
NELSON 43 Mukilteo

4301 78th Street SW Mukilteo, WA

PERMIT SET



12503 Bel-Red Road, Suite 101 Bellevue, WA 98005 p 425 646 1818 f 425 646 4141

NELSON DEVELOPMENT

P.O. BOX 1301 SEAHURST, WA 98062

REVISIONS

10 11 2019 PERMIT SET ISSUE NO. DATE

ITEM

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Digitally signed by Randy Brown Date: 2019.10.10 20:29:01 -07'00'

PROJECT INFORMATION

NELSON 43 4301 78TH STREET SW MUKILTEO, WA

SHEET INFORMATION

release for: BUILDING PERMIT COVER TITLE:

DATE:

DESIGNED BY: DRAWN BY: REVIEWED BY: APPROVED BY: 08 07 19 SHEET NO:

PROJECT NO: 201613.03.003

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—

ABBREVIATIONS

 ANGLE CHBD CL CENTER LINE CI CHANNEL CJT DIAMETER OR ROUND CLG NUMBER OR POUND J PENNY CLK L PERPENDICULAR CLR PENPY CLK L PERPENDICULAR CLR PLATE CONCRETE CONCRETE CONC AC ACOUSTICAL OR ASPHALT CNTR CONCRETE CONC AC ACOUSTICAL PANEL CONC ACT ACOUSTICAL TILE CONN ADD ADDITIVE CONSTR ADJ ADJACENT CORR ADJT ADJUSTABLE CPT AFF ABOVE FINISH FLOOR CT AGG AGGREGATE CTR AL ALUMINUM CTSK AL ALUMINUM CTSK	CHALK BOARDENCCAST IRONCONTROL JOINTEWCCEILINGEQCONSTRUCTION JOINTEQFCAULKINGCLEARESECONCRETE MASONRYESTUNITEXFCOUNTEREXCLEANOUTEXSCOLUMNEXFCONCRETEEXTCONNECTIONFACONSTRUCTIONFBCCONTINUOUSFBCCORRIDORFBCCORRIDORFBCCORRIDORFBCCONTINUOUSFBCCONTINUOUSFBCCONTINUOUSFBCCONTINUOUSFBCCONTINUOUSFBCCONTINUOUSFBCCOUNTER SINKFCTCUBIC YARDFDDEEP, DEPTHFDNDOUBLEFEDEPARTMENTFECDETAILFINDIAGONALFLDIMENSIONFLGDOWNFOFDOOR OR DRAINFORDOWN POOFINGFORDOWN POUTFORDOWNSPOUTFORDAMPPROOFINGFORDAMPPROOFINGFORDOWNSPOUTFORDAMPROOFINGFORDOWN POUTFORDAMPROOFINGFORDOWN POUTFORDAMERERFURDAMINGFOREACHEXPANSION JOINTEXTERIOR INSULATEDFTGFINISH SYSTEMFURELEVATORGAL	OR ENCLOSEDCELECTRIC WATER COOLER EQUALPTEQUIPMENT EMERGENCY SHOWER/EWEYE WASHIESTIMATEHEXHAUST EXPANSIONSTEXISTINGDEXPOSEDIEXTERIOR FIRE ALARM FLAT BARDFIBER BOARDDFURNISHED BY OTHERSCFURNISHED BY OTHERSCFURNISHED BY CONTRACTOR INSTALLED BY CONTRACTORTYFACTORY FLOOR DRAINVFOUNDATION FIRE EXTINGUSHERCFIRE EXTINGUSHERCFIRE EXTINGUSHERCFLASHINGJORFLUORESCENTCFACE OF CONCRETEFACE OF FINISHCFURNISH BY OWNER INSTALL BY CONTRACTOROFURNISH BY OWNER INSTALL BY OWNERSFACE OF STUDMFREEZE PROOF WALL HYDRANT FULL SIZE FEETGFOOTINGRRFURRINGIFUTURE GAUGE	GB GL GLBM GC GND GWB GYP HB HBD HC HDR HDWD HDWE HM HORIZ HR HT HTG HVAC HWH ID IG IN INCL INSUL INSUL INT INV JAN JST JT KO KS LAB LAM LAV LB LF L G LH LL LMS	GRAB BAR GLASS OR GLAZING GLU-LAM BEAM GENERAL CONTRACTOR GROUND GYPSUM WALL BOARD GYPSUM HOSE BIB HARD BOARD HOLLOW CORE OR HAND DRYER HEADER HAND DRYER HEADER HARD WOOD HARDWARE HOLLOW METAL HORIZONTAL HOUR HEIGHT HEATING/VENTILIATION/ AIR CONDITIONING HOT WATER HEATER INSIDE DIAMETER/ DIMENSION INSULATED GLASS INCH INCLUDE INSULATION INTERIOR INVERT JANITOR JOIST JOINT KNOCK OUT KNEE SPACE LABORATORY LAMINATE LAVATORY LAG BOLT LINEAL FOOT LONG, LENGTH LAMINATED GLASS LEFT HAND LIVE LOAD LIQUID MARKING	LT LWC MAS MATL MAX MB MDO MDF MECH MEMB MTL MEZZ MFR MH MIN MIR MIN MIR MISC MO MTD MULL N NIC NO/# NOM NRC NTS OA OBS OC OD OH OPH OPH OPH OPH OPH OPH OPH OPH PERP PL PLAM PLAS PWD	SURFACE LIGHT LIGHT WEIGHT CONCRETE MASONRY MATERIAL MAXIMUM MACHINE BOLT MEDIUM DENSITY OVERLAY MEDIUM DENSITY FIBERBOARD MECHANICAL MEMBRANE METAL MEZZANINE MANUFACTURER MAN HOLE MINIMUM MIRROR MISCELLANEOUS MASONRY OPENING MOUNTED MULLION NORTH NOT IN CONTRACT NUMBER NOMINAL NOISE REDUCTION COEFFICIENT NOT TO SCALE OVERALL OBSCURE ON CENTER OUTSIDE DIAMETER/ DIMENSION OVERHEAD OPPOSITE HAND OPENING OPPOSITE HAND OPENING OPPOSITE HAND OPENING OPPOSITE BOARD PERPENDICULAR PLATE OR PROPERTY LINE PLASTIC LAMINATE PLASTER PLYWOOD	PNL PNT POL PR PRCST PSF PSI PT PTD PTD/R PTD/R PVC PVMT QT R RA RA RA RA RA RA RA RB R&S RD RD/O REBAR REF REFR REFR REFR REFR REFR REFR REF	PANEL PAINT POLISH PAIR PRECAST POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH PRESSURE TREATED PAPER TOWEL DISPENSER PAPER TOWEL DISPENSER AND RECEPTACLE PARTITION PAPER TOWEL RECEPTACLE POLYVINYL CHLORIDE PAVEMENT QUARRY TILE RISER RETURN AIR RADIUS RESILIENT BASE ROD & SHELF ROOF DRAIN ROOF DRAIN OVERFLOW REINFORCING BAR REFERENCE, REFLECTED REFRIGERATOR REINFORCED REQUIRED REVISION RIGHT HAND OR ROBE HOOK RESILIENT ROUGH-IN ONLY ROOM ROUGH OPENING RESILIENT/RUBBER TILE RUBBER RAIN WATER LEADER SOUTH SOLID CORE SEAT COVER DISPENSER	SCHD SD SECT SF SHT SHTG SIG SIM SK SLR SND SNR SNT SPEC SPGL SQ SS SST SSK STA STC STN STD STL STOR STC STN STD STL STOR STC STN STD STL STOR STC STN STD STL STOR STL STD STL STD STL STOR STL STD STL STD STL STOR STL STD STL STD STL STOR STL STD STD STD STD STD STD STD STD STD STD	SCHEDULE SOAP DISPENSER OR STORM DRAIN SECTION SQUARE FEET SHEET SHEATHING SOLAR INSULATED GLAZING SIMILIAR SINK SEALER SANITARY NAPKIN DISPENSER SANITARY NAPKIN RECEPTACLE SEALANT SPECIFICATION SPANDREL GLASS SQUARE SOLID SURFACING STAINLESS STEEL SERVICE SINK STATION SOUND TRANSMISSION CLASS STAIN STANDARD STEEL STORAGE STRUCTURAL SUSPENDED SHEET VINYL SYMMETRICAL SPECIAL WALL COVERING TREAD TOWEL BAR TOP & BOTTOM TONGUE & GROOVE TEMPERED GLASS THICK THRESHOLD TEMPERED INSULATED GLAZING TACK BOARD TOP OF	TOC TOP TOS TOSL TOW TPD TPH TPTN TS TV TYP UL UON UR VAR VCT VENT VENT VENT VENT VENT VENT VEST VNR VR VWC W/ W/ W/ W/ W/ W/ W/ W/ W/ W/ W/ W/ W/	TOP OF CONCRETE TOP OF PAVEMENT TOP OF STEEL TOP OF SLAB TOP OF WALL TOILET PAPER DISPENSER TOILET PAPER HOLDER TOILET PARTITION TUBULAR STEEL TELEVISION TYPICAL UNDERWRITERS LABORATORY UNLESS OTHERWISE NOTED URINAL VARIES VINYL COMPOSITION TILE VENTILATOR VERTICAL VESTIBULE VENEER VAPOR RETARDER VINYL WALL COVERING WEST WITH WITHOUT WATER CLOSET WOOD WINDOW WIDE FLANGE WIRE GLASS WIRE MESH WATER PROOF WORKING POINT WATER RESISTANT WAINSCOT WEIGHT WELDED WIRE FABRIC TRANSFORMER YARD
A COLUMN LINE/GRID LINE A01 DOOR NUMBER (1) KEY NOTE (1) WALL TYPE	REVISION/ CLO MATCH LINE WORK POINT, POINT OR DAT	CONTROL TUM POINT	1 A9.1 1 A5.1 <u>OFFICI</u> 101	INTERIOR ELEVATION IDENTIFICATION/ SHEET NUMBER		NORTH PROJECT N A01 EXTERIOR OPENING I EQ-1 EQUIPMENT IDENTIFICAT PNT-1 FINISH IDENTIFICAT	GLAZED NO. T TION	1. ENERGY CO WAREH OFFICE COMPL 2. INSULATION ROOF WALL PERIMI SKYLIG SECTIC ROLL-	DE COMP IOUSE AR AREAS: IANCE ME REQUIRE INSULATIO INSULATIO ETER SLAE	PLIANCE: EAS: SEMI-HEAT CONDITIONE ETHOD: COMPONEN MENTS: DN: RIGID CONT NI: NONE B INSULATION: NONE U=0.5 RS: R-10 2S: R-10	.D T	WER DECK: R-30

ENTRANCES: U=0.60 3. SHADING COEEFICIENTS SKYLIGHTS: SHGC=0.40 MAX. FIXED WINDOWS: SHGC=0.40 MAX. SHGC=0.40 MAX. ENTRANCES:

ZONING INFORMATION

ZUNING INFORMA	ATION
1. Jurisdiction:	EVERETT, WA
2. Present Zoning:	P1 — Planned Industrial RD — Single—Family residential
3. Projected site use requires:	NO ACTION
4. Use Zone Adjacent Lots:	N <u>RD</u> E <u>P1</u> S <u>P1</u> W <u>RD</u>
 Special Regulations Applicable: A. Shoreline Management Act Setback required B. Flood Plain or Waterway C. Airport open use/glide pat D. Storm Water Retention/Rur E. Fire Lane Requirements 	
	Yard <u>10'</u> Rear Yard <u>20'</u> le Yard <u>10'</u> E Side Yard <u>10'</u>
7. Easements, Vacations, restrictiv SEE CIVIL DRAWINGS	e convenants as applicable:
8. Parking Building Shell (Office space ar	
Warehouse/Office Parking Stalls Required	55,820 SF 0.5 Stall/1,000 SF 30 4 Accessible (1 Van) 60 Parking 64
Parking Stalls Provided	
	SIZE: 8'-6" x 19'-0"
Compact:	SIZE: 8'-0" x 16'-0"
9 Total Site Area	176 141 SF 4 04

9. Total Site Area		176,141 SF	4.04 Acre
Parking Area:		29,564 _ SF	
Parking Landscaping Required:	10%	2,956_ SF	
Landscaping Provided:	16%	4,751 SF	

_ Stalls

Total

_ Total

GENERAL NOTES

- 1. DIMENSIONS ARE TO FACE OF STUD, CONCRETE OR CENTER LINE OF COLUMN, UNLESS OTHERWISE NOTED.
- 2. DO NOT SCALE DRAWINGS; DIMENSIONS GOVERN.
- 3. VERIFY ALL EXISTING CONDITIONS, DIMENSIONS, DETAILS, ETC. NOTIFY ARCHITECT OF ANY AND ALL DISCREPANCIES PRIOR TO PROCEEDING WITH THE WORK.
- 4. WHEN CONSTRUCTION DETAILS ARE NOT SHOWN OR NOTED FOR ANY PART OF THE WORK, DETAILS SHALL BE THE SAME AS FOR OTHER SIMILAR WORK. IF QUESTIONS CANNOT BE RESOLVED IN THIS MANNER, CONTACT THE ARCHITECT.
- 5. VERIFY ELEVATIONS & LOCATIONS TO BE JOINED BEFORE CONSTRUCTION. CONTACT ARCHITECT IF THEY DIFFER FROM THOSE SHOWN ON DRAWINGS.
- 6. ALL ROUGH-INS TO BE APPROVED PRIOR TO FRAMING INSPECTION.
- 7. MECHANICAL, ELECTRICAL, AND PLUMBING ARE BIDDER DESIGN. PORTIONS OF THIS WORK SHOWN ON DRAWINGS IS FOR DESIGN INTENT OR FOR COORDINATION ONLY.
- 8. MECHANICAL DESIGN/BUILD CONTRACTOR SHALL PROVIDE A MECHANICAL VENTILATION SYSTEM CAPABLE OF SUPPLYING THE MINIMUM OUTDOOR AIR QUANTITIES SPECIFIED IN THE 2015 IMC WITH WASHINGTON AMENDMENTS.
- 9. ELECTRICAL DESIGN/BUILD CONTRACTOR SHALL PROVIDE AND INSTALL MEANS OF EGRESS ILLUMINATION AND ILLUMINATED EXIT SIGNS PER IBC SECTIONS 1006 AND 1011.
- 10. DEFERRED SUBMITTALS: STRUCTURAL JOISTS FIRE ALARM AND SPRINKLERS

CODE INFORMATION

Code Edition: 2015 IBC, 2015 IFC, 2015 IMC, 2015 UPC, 2015 WSEC (WAC 51-11), ICC/ANSI A117.1-2009; WASHINGTON STATE AMENDMENTS TO THE ABOVE (WAC 51-50).
Scope of Work: Construct a 55,820 square foot industrial facility utilizing site cast concrete tilt-up wall panels, a wood roof supported on composite steel joists and girders with wood nailers and HSS columns. The facility is single story with a minimum clear height of 24'-0" above finished floor. There will be a total of (20) 9' X 10' dock high doors and (4) 12' X 14' grade access doors. The facility is fully sprinklered with an ESFR system.
Amended by Local Jurisdiction Yes <u>X</u> No Date
X Basic Wind Speed 110 MPH Addition Seismic Use Group 1 Alteration Occupancy Category 2 Repair Site/Soil Classification D Snow Load 25 PSF
BUILDING CLASSIFICATION
A. Occupancy Classification (Section 302) <u>B / S-1</u>
B. Type of Construction (Section 602) 1. Type of Construction <u>III-B</u> 2. Automatic Sprinklers Provided Yes <u>X</u> No <u></u>
C. Location of Property (602.1) SEE SHEET A1.1
Distance to Openings Openings Fire Resistance Property Line Permitted Protected of Exterior Wall (Table 705.8) (Table 705.8) (Table 601 & 602)
North 88' Feet Yes No Yes No X East 138' Feet Yes No Yes No X South 91' Feet Yes No Yes No X West 27' Feet Yes No Yes No X (Other) - Feet Yes No Yes No X
D. Building Area (Section 506)
 At , Tabular Area (Table 506.2) S-1 OCCUPANCY Frontage Increase (506.3) W (Width of Open Space per 506.3.2) = 30 FT If (Increase due to Frontage) =
100[(F/P)-0.25](W/30) = 75 % 3. NS, Tabular Area (Table 506.2) 17,500 SF
4. A _a , Allowable Area (506.2.1) A _t + (NS x I _f)= 83,125 SF
5. Unlimited Building Area (507) — Section 507.3 — Section 507.4 — Section 507.5 — Section 507.5 — Section 507.6 — Section 507.7 — Section 507.8 — Section 507.9 — Section 507.10 — Section 507.11 — Section 507.12 — Section 507.13
6. Actual Floor Area: 1st Flr: _ 55,820 SF Mezz:0 SF Total: 55,820 SF
E. Allowable Height and Number of Stories (Table 504.3 & 504.4)

 75
 Feet
 3
 Stories

 32'-0"
 Feet
 1
 Stories
 Tabular Building Height Allowed
 Actual Building Height

- ------ X ------ FENCE LINE

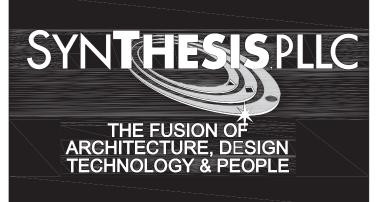
INDEX OF DRAWINGS

<u>ARCHITECTURAL</u>

- COVER G1.1 GENERAL INFORMATION
- A1.1 SITE PLAN A1.2 SITE PLAN DETAILS
- A2.1 FLOOR PLAN
- A2.2 UPPER FLOOR PLAN A2.3 ROOF PLAN
- A3.1 WINDOW TYPES, WALL TYPES, SCHEDULES A4.1 EXTERIOR ELEVATIONS
- A4.3 WALL SECTIONS
- A4.4 WALL SECTIONS A8.1 EXTERIOR DETAILS

<u>STRUCTURAL</u>

S1.0 GENERAL NOTES S1.01 SPECIAL INSPECTIONS & ABBREVIATIONS S1.1 FOUNDATION SECTIONS S2.0 FOUNDATION PLAN S2.1 ROOF FRAMING PLAN S3.0 SCHEDULES & DIAGRAMS S5.0 ROOF FRAMING SECTIONS S5.1 ROOF FRAMING SECTIONS & DIAGRAMS S6.0 PANEL CONNECTIONS, PANEL DETAILS & PANEL KEY S6.1 PANEL ELEVATIONS S6.2 PANEL ELEVATIONS S6.3 PANEL ELEVATIONS S6.4 PANEL ELEVATIONS S6.5 PANEL ELEVATIONS



12503 Bel-Red Road, Suite 101 Bellevue, WA 98005 p 425 646 1818 f 425 646 4141



P.O. BOX 1301 SEAHURST, WA 98062

PROJECT INFORMATION

PROJECT SCOPE:	NEW BUILDING
PROJECT ADDRESS:	4301 78TH STREET SW MUKILTEO, WA 98275
BUILDING OWNER:	NELSON DEVELOPMENT P.O. BOX 1301 SEAHURST, WA 98062
PROPERTY TAX NO.:	2841000300100, 28041000300400, 2841000300500 & 28041000300600
ARCHITECT:	SYNTHESIS PLLC 12503 BEL-RED ROAD, SUITE 101 BELLEVUE, WA 98005 (425) 646–1818 CONTACT: RANDY BROWN
STRUCTURAL:	SHUTLER CONSULTING ENGINEERS 12503 BEL-RED ROAD, SUITE 100 BELLEVUE, WA 98005 (425) 450-4075 CONTACT: JOHN HEADLAND

1 10 11 2019 PERMIT SET ISSUE NO. DATE

7154 REGISTERED ARCHITECT

ITEM

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REVISIONS

Digitally signed by Randy Brown Date: RANDALL LEE BROWN STATE OF WASHINGTON 2019.10.10 20:29:36 -07'00'

PROJECT INFORMATION

NELSON 43 4301 78TH STREET SW MUKILTEO, WA

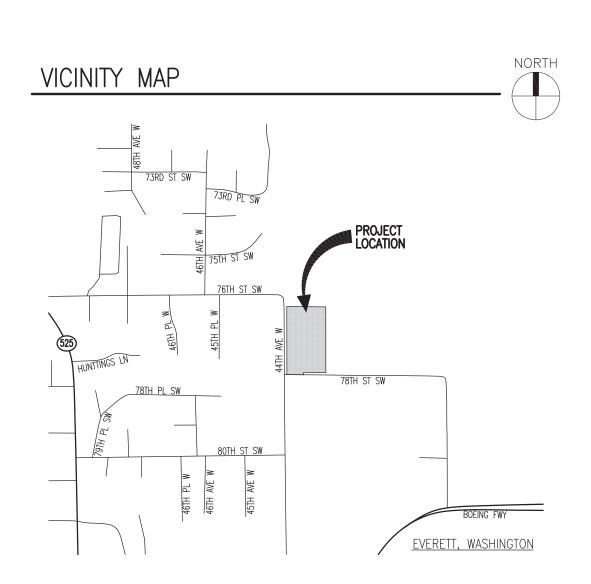
S	SHEET	INFOF	۲M	AΤ	$ \mathbf{O} $	N	
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release for: BUILDING PERMIT CODE INFORMATION, SITE PLAN TITLE:

DESIGNED BY: DRAWN BY: APPROVED BY: REVIEWED BY: date: 08 07 19 G1.1 SHEET NO: PROJECT NO: 201613.03.003

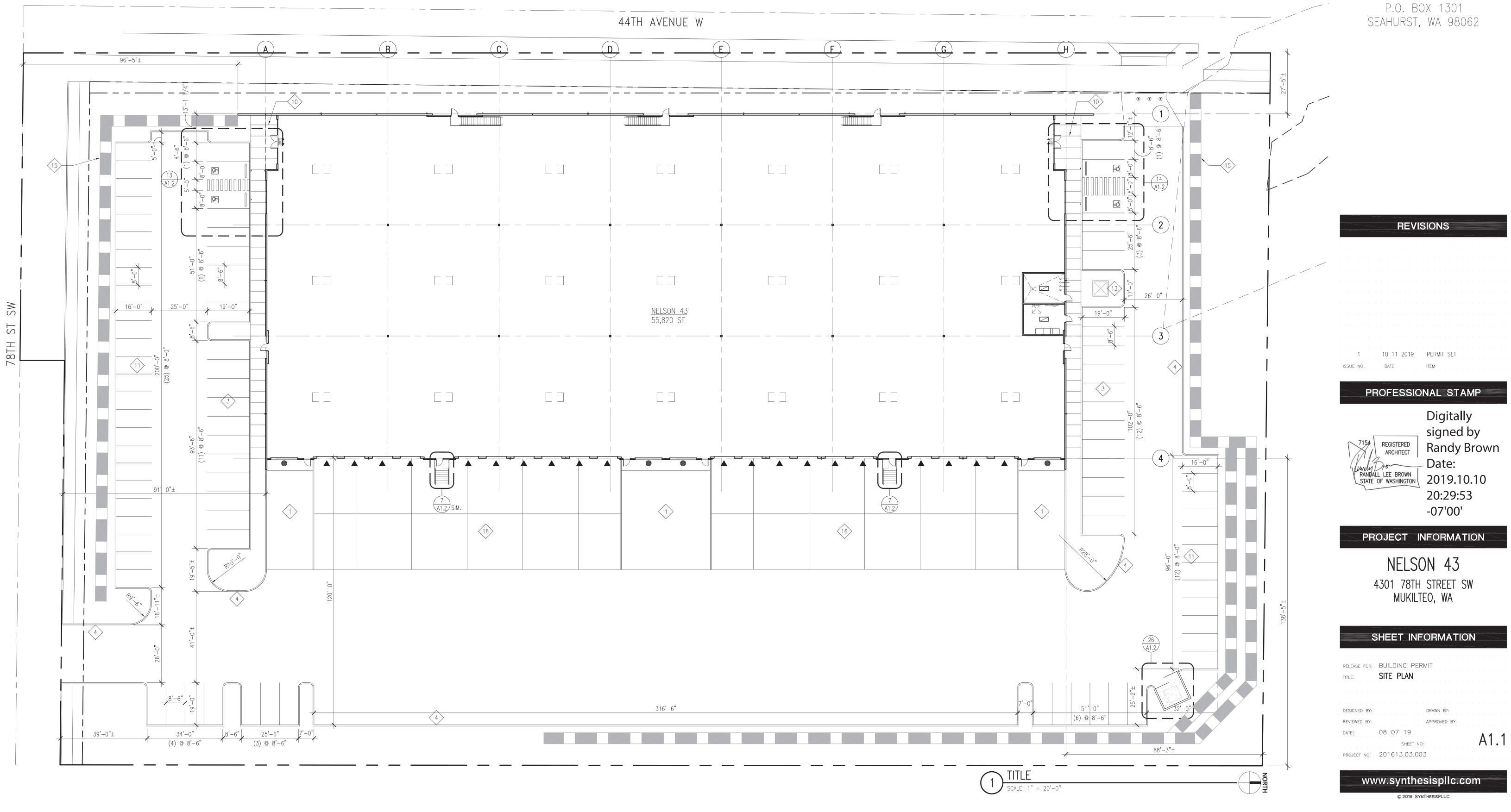
LEGAL DESCRIPTION

SEC 10 TWP 28 RGE 04RT-10) W1/2 NW1/4 NW1/4 SW1/4 LESS N 100 FT & LESS S 231 FT & LESS W 208.71 FT



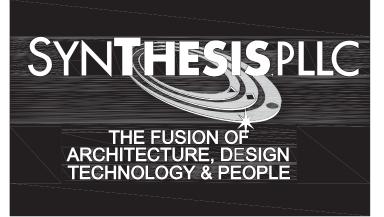
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SITE KEYNOTES		7'–0" HIGH CONC. TRASH ENCLOSURE	(10)	4" THICK, 5'—O"w (UON) CONC WALK, MAX SLOPE 1v:20h.	(15)	ACCESSIBLE PATH OF TRAV
NOTE: NOT ALL KEYNOTES ARE	USED	SEE A1.2		MAX SLOPE 1v:20h.	$\sqrt{10}$	PUBLIC WAY
AC PVMT RAMP, 1:12 SEE CIVIL DWGS.	MAX SLOPE,	ACCESSIBLE PARKING SIGN, SEE DET 11/A1.2.		COMPACT PARKING STALL W/ PAINTED "COMPACT" DESIGNATION.	16	50' CONCRETE ARPRON
2 CONC LANDING, 5'-0"	W x 5'-0"L MIN. 7	ACCESSIBLE PARKING STALL W/ PAINTED INTERNATIONAL SYMBOL OF ACCESS. M/ SLOPE 1v:48h. SEE 16/A1.2.) AX 12	PROVIDE SIGN W/ THE INTERNATIONAL SYMBOL OF ACCESS, ATTACH TO GLASS ADJACENT TO DOOR.	(17)	RETAINING WALL, SEE CIVIL
3 PAINT STRIPING ON FL PAVEMENT.	LOOR OR	VAN ACCESSIBLE PARKING STALL W/ PAINTED INTERNATIONAL SYMBOL OF ACCESS. MAX SLOPE 1v:48h. SEE 16/	A1.2.	APPROXIMATE ELECTRICAL TRANSFORMER LOCATION	18	BICYCLE RACK (FOR 2 OR BICYCLES)
4 EXTRUDED CONC CURI 24" RAD. OUTSIDE CO USE THICK-FACE CUR	RNERS TYP UÓN. $\langle 9 \rangle$	ACCESSIBLE RAMP, MAX SLOPE 1v:12h, MAX RISE 6" MIN LENGTH 6'-0", W/ NON-SLIP SURFACE.	14	APPROXIMATE GAS METER LOCATION		



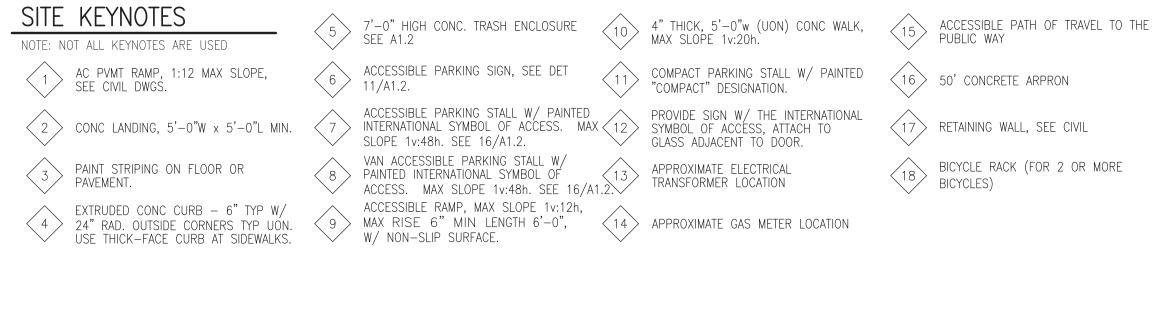
LL, SEE CIVIL (FOR 2 OR MORE

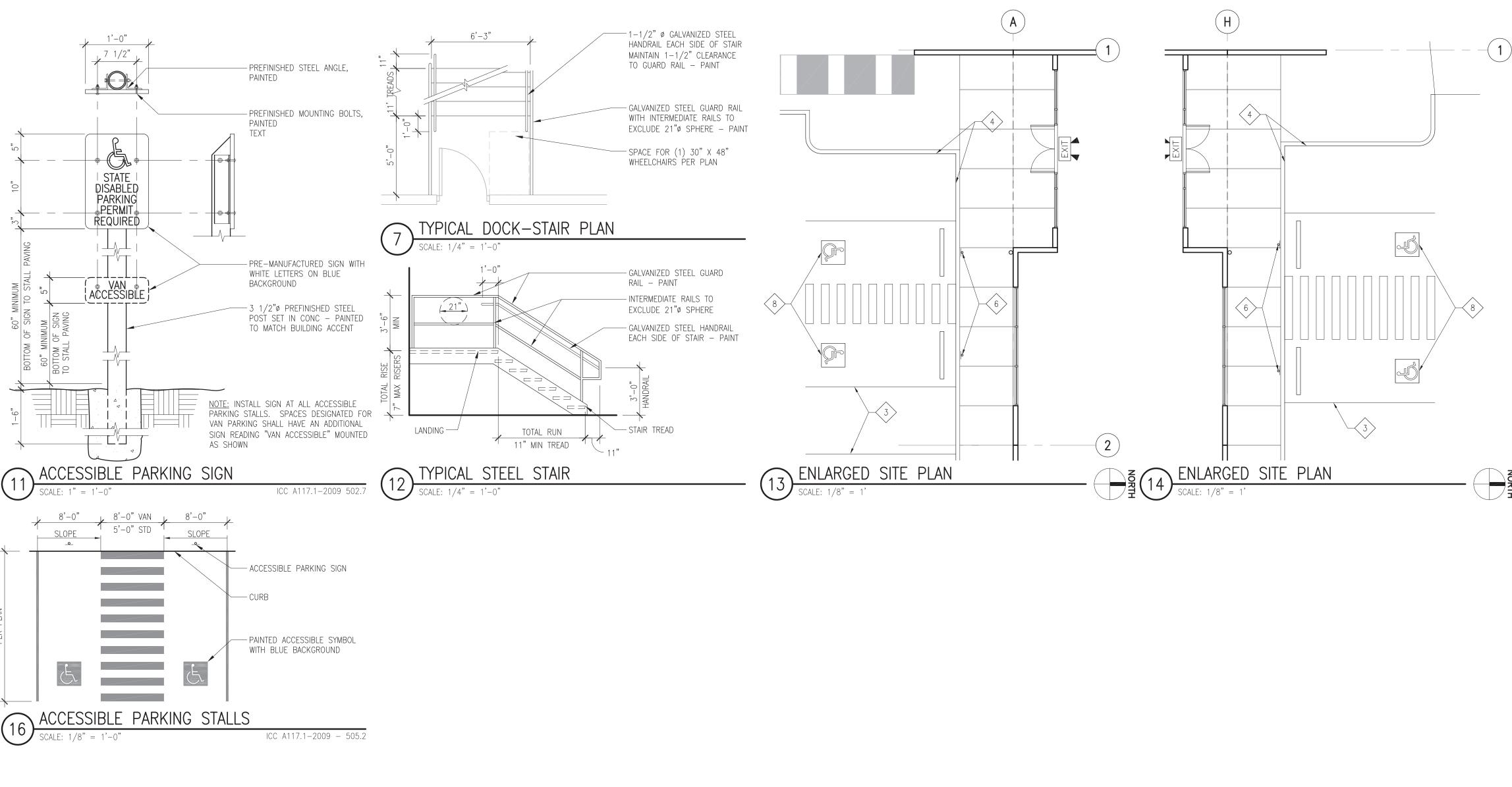
PATH OF TRAVEL TO THE

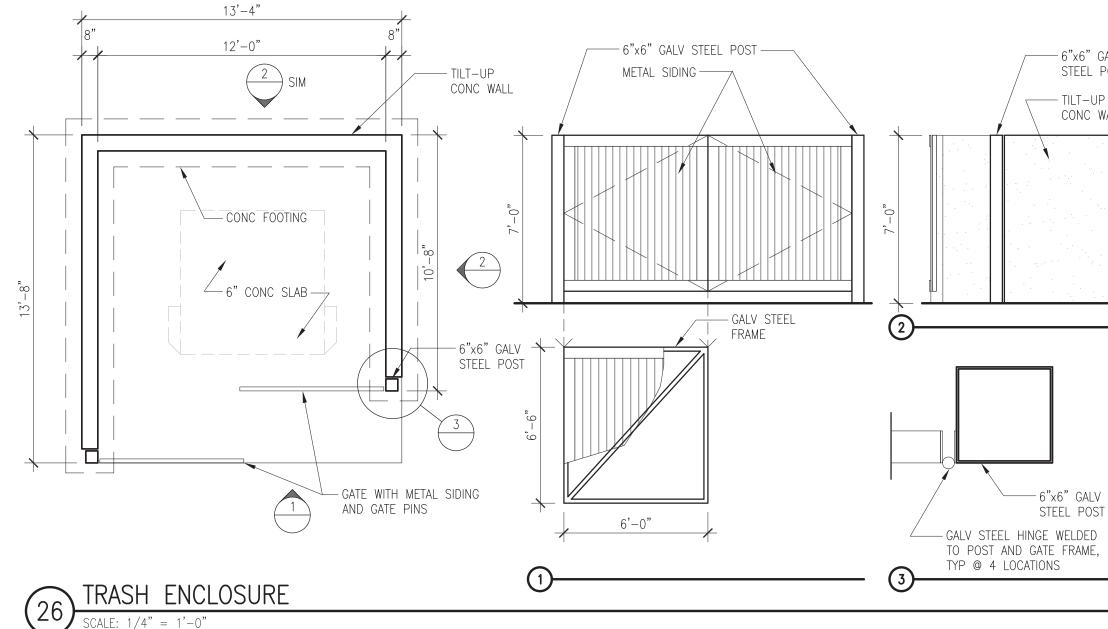


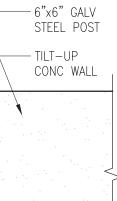
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NELSON DEVELOPMENT



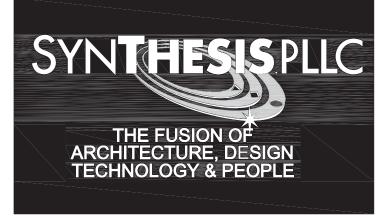






STEEL POST

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ITEM

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PROJECT INFORMATION

NELSON 43 4301 78TH STREET SW MUKILTEO, WA

SHEET INFORMATION

release for: BUILDING PERMIT TITLE:

SITE PLAN DETAILS

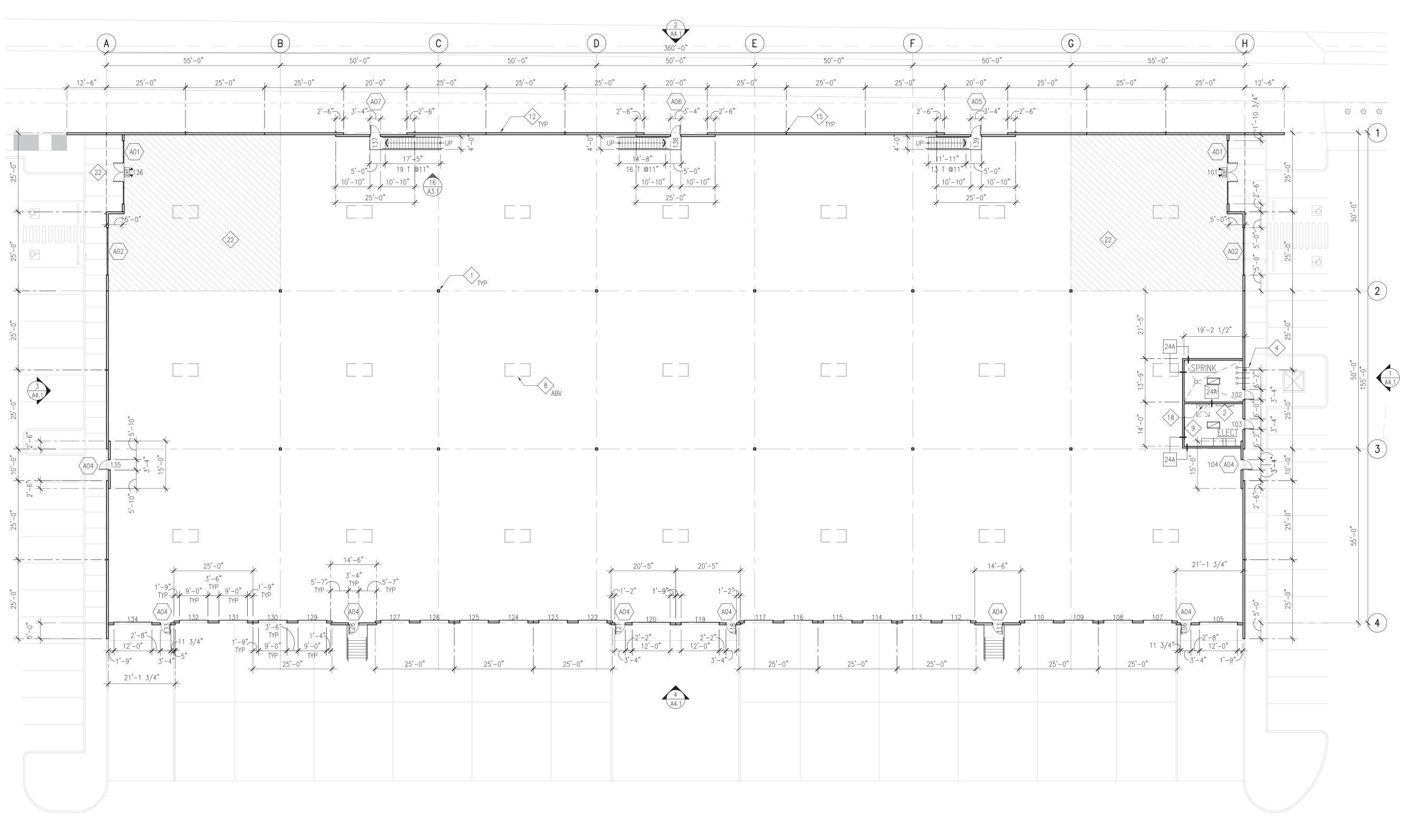
DESIGNED BY: REVIEWED BY: 08 07 19 DATE:

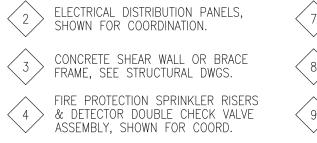
DRAWN BY: APPROVED BY: SHEET NO:

A1.2

PROJECT NO: 201613.03.003

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PLAN KEYNOTES

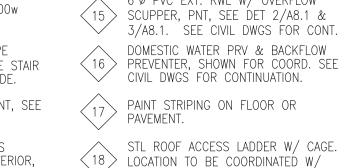
NOTE: NOT ALL KEYNOTES ARE USED

1 STEEL COLUMNS TYP, SEE STRUCTURAL DWGS.

- 9 4'x 8'x 3/4" PLYWOOD TELEPHONE BOARD.
- 4' X 8' (UON) ACRYLIC TRIPLE DOME84' X 8' (UON) ACRYLIC TRIPLE DOME88999</t
- FOR CONTINUATION. TOILET ROOM (TOILET ROOM N.I.C.)
- 6 SANITARY SEWER, SHOWN FOR COORDINATION, SEE CIVIL DRAWINGS

FOR CONTINUATION.

- GAS METER, SHOWN FOR 5 COORDINATION, SEE CIVIL DRAWINGS 10 HIBAY HID LIGHT FIXTURE, 400w METAL HALIDE LAMP.
 - 1 1/2"Ø NOM. GALV STL PIPE HANDRAIL. TOP @ 34" ABOVE STAIR NOSING OR RAMP TYP EA SIDE. 12 SITE-CAST CONC PANELS, PNT, SEE STRUCTURAL DWGS. 6"Ø RD & 6"Ø SCHD 40 ABS 13 PLASTIC RWL, ROUTE TO EXTERIOR, SEE CIVIL DWGS FOR CONTINUATION. 14 STL ANGLE RWL PROTECTOR, TYP @ TRUCK COURT, SEE DETAIL 5/A8.1







18 LOCATION TO BE COORDINATED W/ STRUCTURAL FRAMING LOCATIONS. 48"x 48" ROOF ACCESS HATCH, SEE DET 9/A8.1.

3/A8.1. SEE CIVIL DWGS FOR CONT. DOMESTIC WATER PRV & BACKFLOW

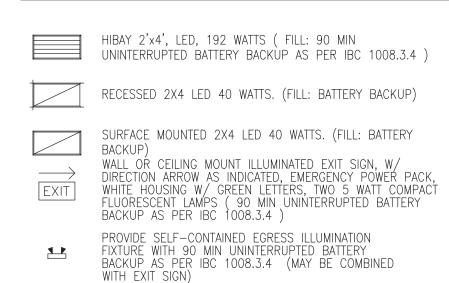
20 PREFABICATED GALV STEEL PAN STAIRS & LANDINGS W/ CONC FILL, 7" MAX RISER, 11" MIN TREAD. ∧ 1 1/2"ø NOM. GALV STEEL PIPE $\langle 21 \rangle$ guard, top @ 42" above landing, STAIR NOSING OR RAMP, PAINT, TYP. CONC SLAB ON VAPOR BARRIER ON 22 4" GRAVEL CAPILLARY BREAK IN ✓ OFFICE NODES (SHOWN SHADED). PROVIDE SIGN W/ INTERNATIONAL SYMBOL OF ACCESS, ATTACH TO GLASS ADJACENT TO DOOR 24 HVAC EQUIPMENT AND PVC WALKWAY PADS BY TENANT

28 PROVIDE 10 MIL VAPOR BARRIER UNDER OFFICE AREAS

26 RECESSED DOCK LEVELER 27 ENTRY CANOPY - SEE STRUCTURAL

 $\langle 25 \rangle$ (2) 30"x48" WHEELCHAIR SPACE.

LIGHT FIXTURE LEGEND





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SHEET INFORMATION

RELEASE FOR: BUILDING PERMIT TITLE:

FLOOR PLAN

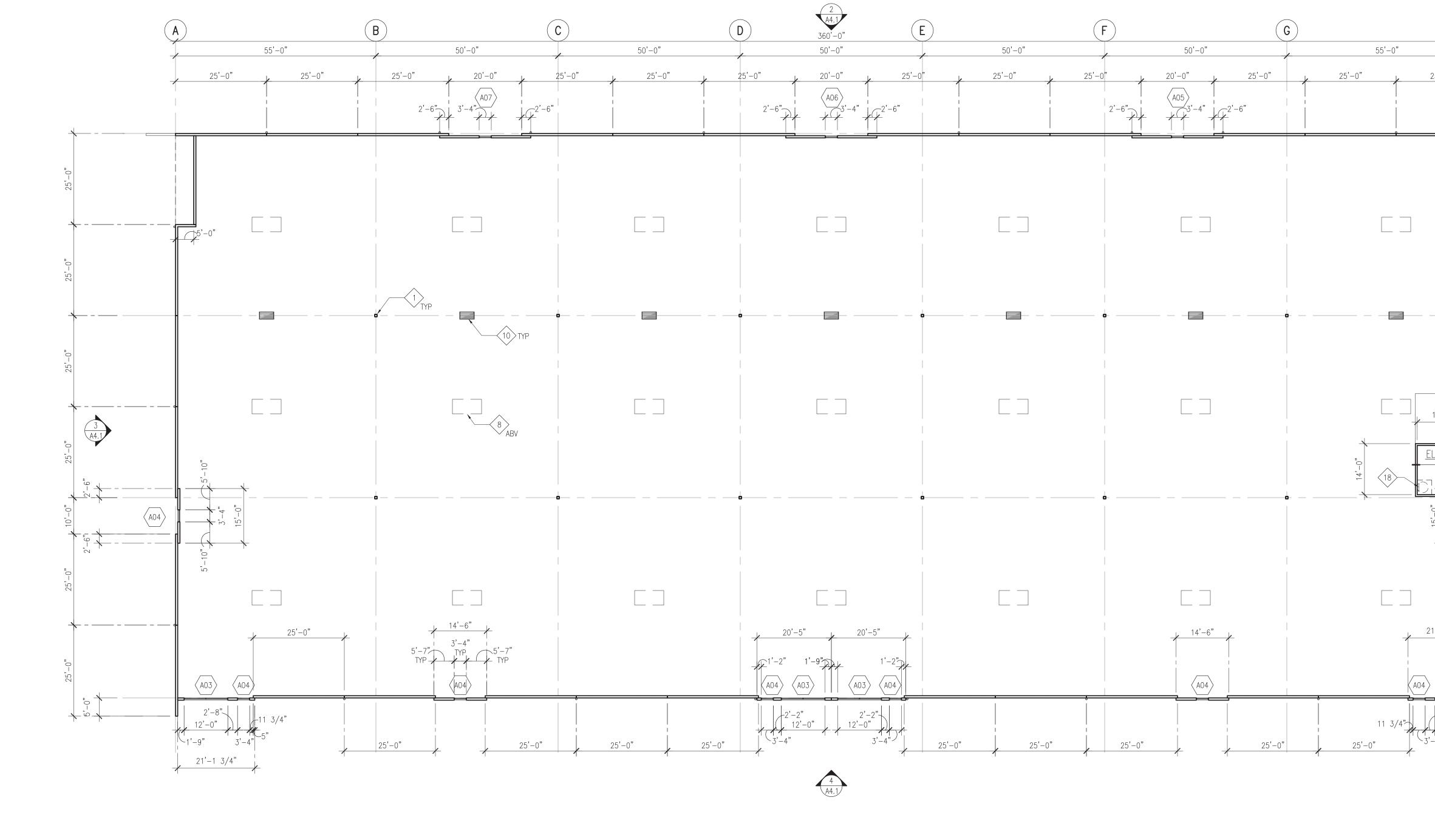
DESIGNED BY: REVIEWED BY: DATE:

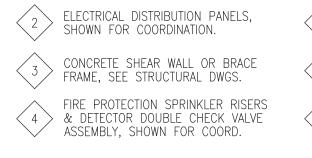
DRAWN BY: APPROVED BY: 08 07 19 SHEET NO:

PROJECT NO: 201613.03.003

A2.1

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PLAN KEYNOTES

NOTE: NOT ALL KEYNOTES ARE USED

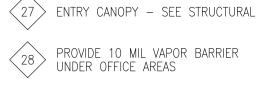
1 STEEL COLUMNS TYP, SEE STRUCTURAL DWGS.

- 9 4'x 8'x 3/4" PLYWOOD TELEPHONE BOARD.
- T STUB UP PLUMBING FOR FUTURE TOILET ROOM (TOILET ROOM N.I.C.) 8 4' X 8' (UON) ACRYLIC TRIPLE DOME SKYLIGHT, SEE DET 8/A8.1.
- FOR CONTINUATION.
- 6 SANITARY SEWER, SHOWN FOR COORDINATION, SEE CIVIL DRAWINGS
- GAS METER, SHOWN FOR 5 COORDINATION, SEE CIVIL DRAWINGS FOR CONTINUATION.
- 14 STL ANGLE RWL PROTECTOR, TYP @ TRUCK COURT, SEE DETAIL 5/A8.1
- 1 1/2"Ø NOM. GALV STL PIPE HANDRAIL. TOP @ 34" ABOVE STAIR NOSING OR RAMP TYP EA SIDE. 12 SITE-CAST CONC PANELS, PNT, SEE STRUCTURAL DWGS. 6"Ø RD & 6"Ø SCHD 40 ABS 13 PLASTIC RWL, ROUTE TO EXTERIOR, SEE CIVIL DWGS FOR CONTINUATION.
- 10 HIBAY HID LIGHT FIXTURE, 400w METAL HALIDE LAMP.



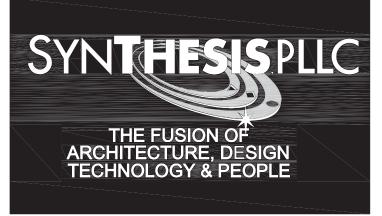


20 PREFABICATED GALV STEEL PAN STAIRS & LANDINGS W/ CONC FILL, 7" MAX RISER, 11" MIN TREAD. 1 1/2"ø NOM. GALV STEEL PIPE GUARD, TOP @ 42" ABOVE LANDING, STAIR NOSING OR RAMP, PAINT, TYP. CONC SLAB ON VAPOR BARRIER ON 22> 4" GRAVEL CAPILLARY BREAK IN OFFICE NODES (SHOWN SHADED). PROVIDE SIGN W/ INTERNATIONAL SYMBOL OF ACCESS, ATTACH TO GLASS ADJACENT TO DOOR 24 HVAC EQUIPMENT AND PVC WALKWAY PADS BY TENANT



26 RECESSED DOCK LEVELER

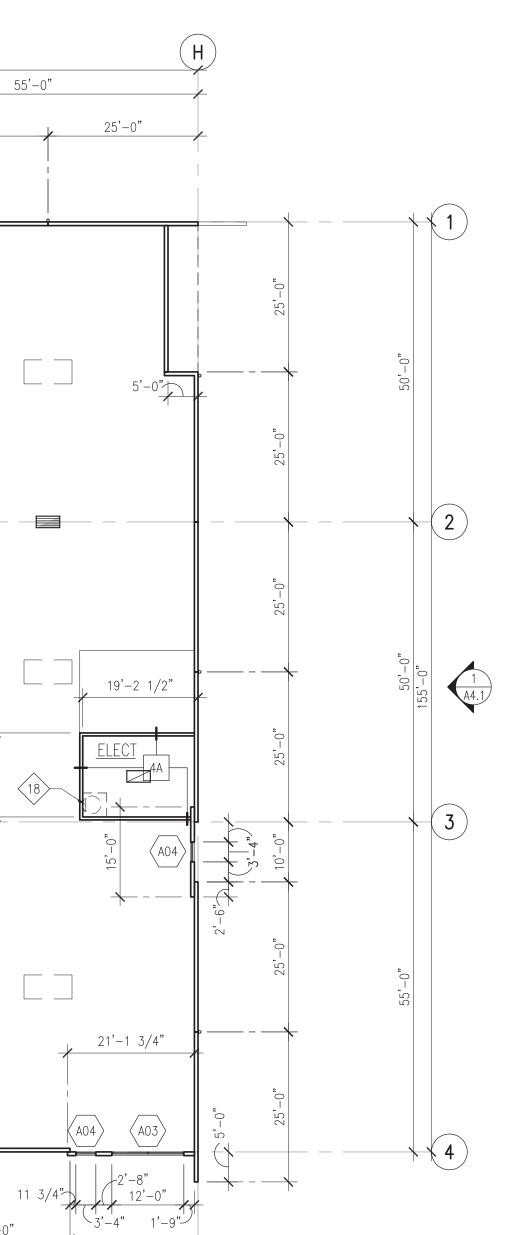
 $\langle 25 \rangle$ (2) 30"x48" WHEELCHAIR SPACE.

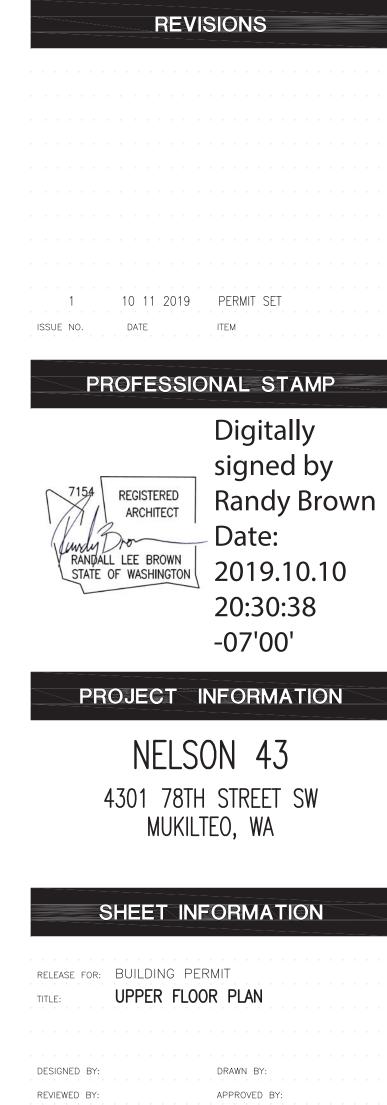


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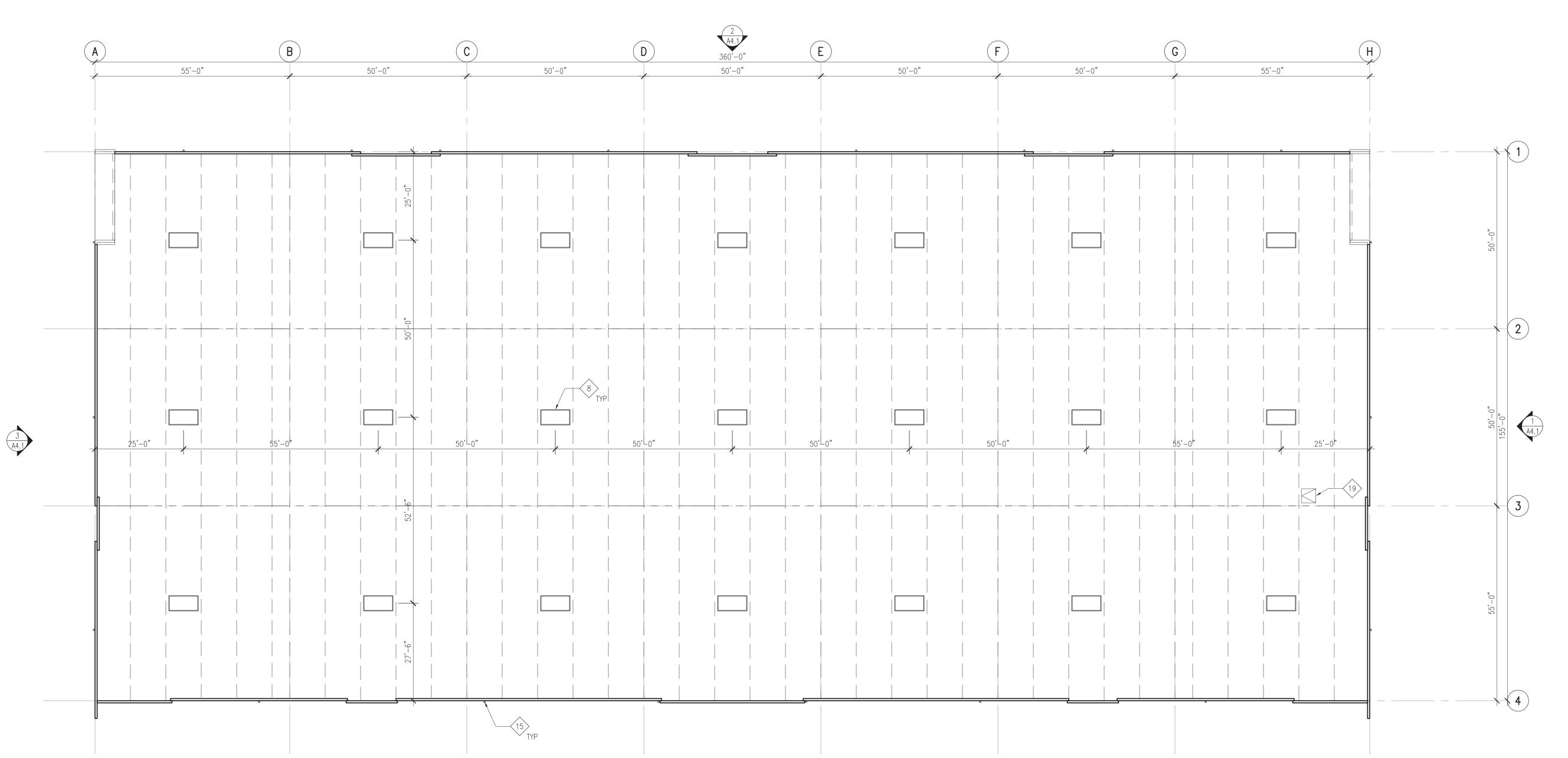
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PROJECT NO: 201613.03.003

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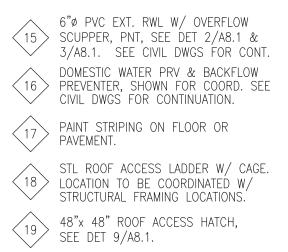
2 ELECTRICAL DISTRIBUTION PANELS, SHOWN FOR COORDINATION. CONCRETE SHEAR WALL OR BRACE FRAME, SEE STRUCTURAL DWGS. 4 FIRE PROTECTION SPRINKLER RISERS & DETECTOR DOUBLE CHECK VALVE ASSEMBLY, SHOWN FOR COORD.

PLAN KEYNOTES

NOTE: NOT ALL KEYNOTES ARE USED

1 STEEL COLUMNS TYP, SEE STRUCTURAL DWGS.

- 9 4'x 8'x 3/4" PLYWOOD TELEPHONE BOARD.
- TOILET ROOM (TOILET ROOM N.I.C.) 8 4' X 8' (UON) ACRYLIC TRIPLE DOME SKYLIGHT, SEE DET 8/A8.1.
- 6 SANITARY SEWER, SHOWN FOR COORDINATION, SEE CIVIL DRAWINGS FOR CONTINUÁTION.
- GAS METER, SHOWN FOR 5 COORDINATION, SEE CIVIL DRAWINGS FOR CONTINUATION.
 - 10 HIBAY HID LIGHT FIXTURE, 400w METAL HALIDE LAMP.
 - 1 1/2"Ø NOM. GALV STL PIPE HANDRAIL. TOP @ 34" ABOVE STAIR NOSING OR RAMP TYP EA SIDE. 12 SITE-CAST CONC PANELS, PNT, SEE STRUCTURAL DWGS. 6"ø RD & 6"ø SCHD 40 ABS 13 PLASTIC RWL, ROUTE TO EXTERIOR, SEE CIVIL DWGS FOR CONTINUATION. 14 STL ANGLE RWL PROTECTOR, TYP @ TRUCK COURT, SEE DETAIL 5/A8.1





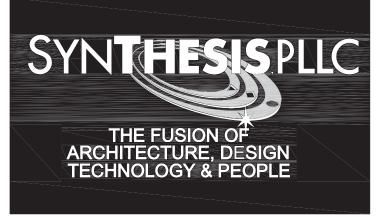


20 PREFABICATED GALV STEEL PAN STAIRS & LANDINGS W/ CONC FILL, 7" MAX RISER, 11" MIN TREAD. 21 1/2"Ø NOM. GALV STEEL PIPE GUARD, TOP @ 42" ABOVE LANDING, STAIR NOSING OR RAMP, PAINT, TYP. CONC SLAB ON VAPOR BARRIER ON $\langle 22 \rangle$ 4" gravel capillary break in ✓ OFFICE NODES (SHOWN SHADED). 23 PROVIDE SIGN W/ INTERNATIONAL SYMBOL OF ACCESS, ATTACH TO GLASS ADJACENT TO DOOR 24 HVAC EQUIPMENT AND PVC WALKWAY PADS BY TENANT

28 PROVIDE 10 MIL VAPOR BARRIER UNDER OFFICE AREAS

26 RECESSED DOCK LEVELER 27 ENTRY CANOPY – SEE STRUCTURAL

(2) 30"x48" WHEELCHAIR SPACE.



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signed by Randy Brown Date: 20:30:57 -07'00'

PROJECT INFORMATION

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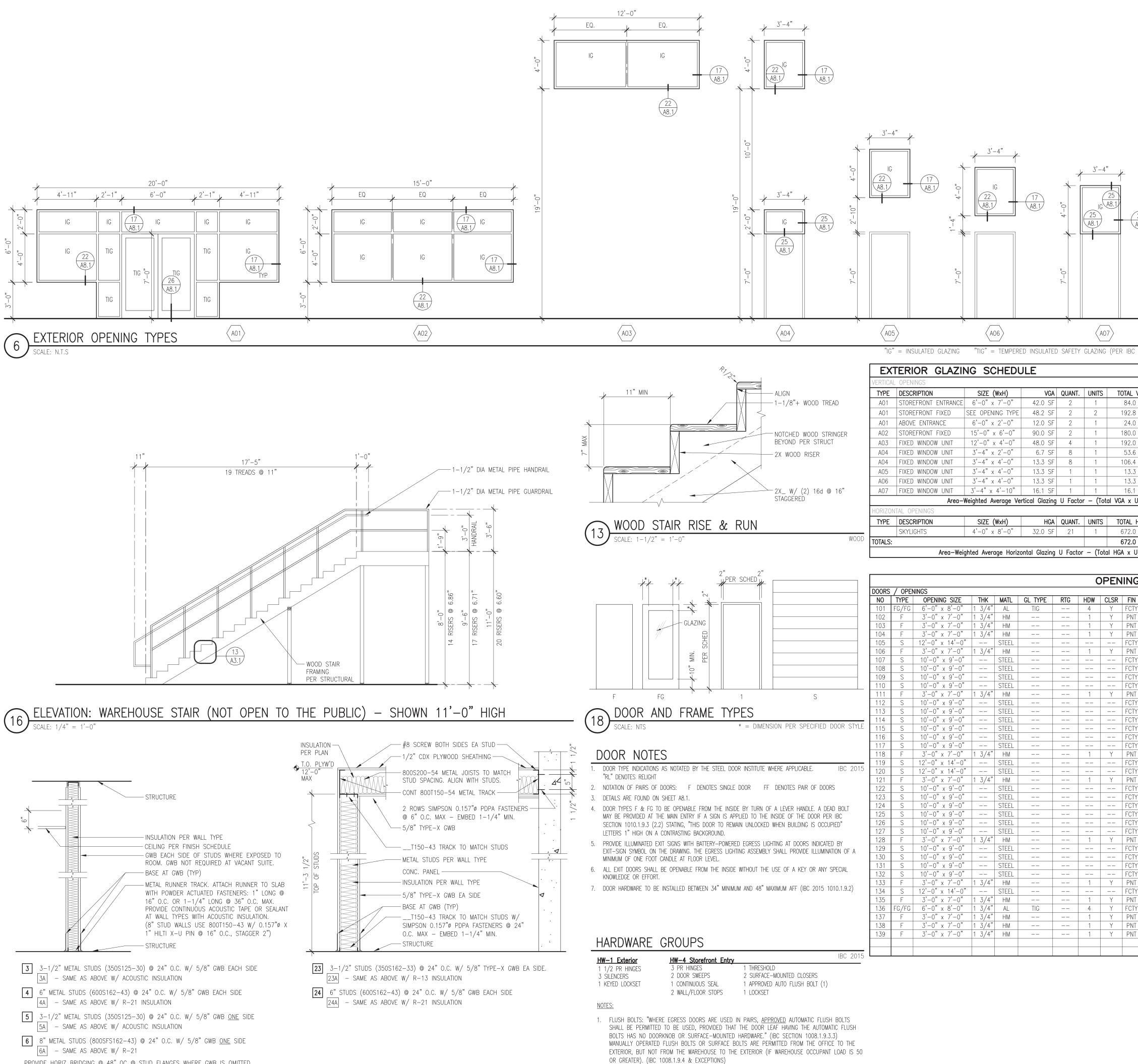
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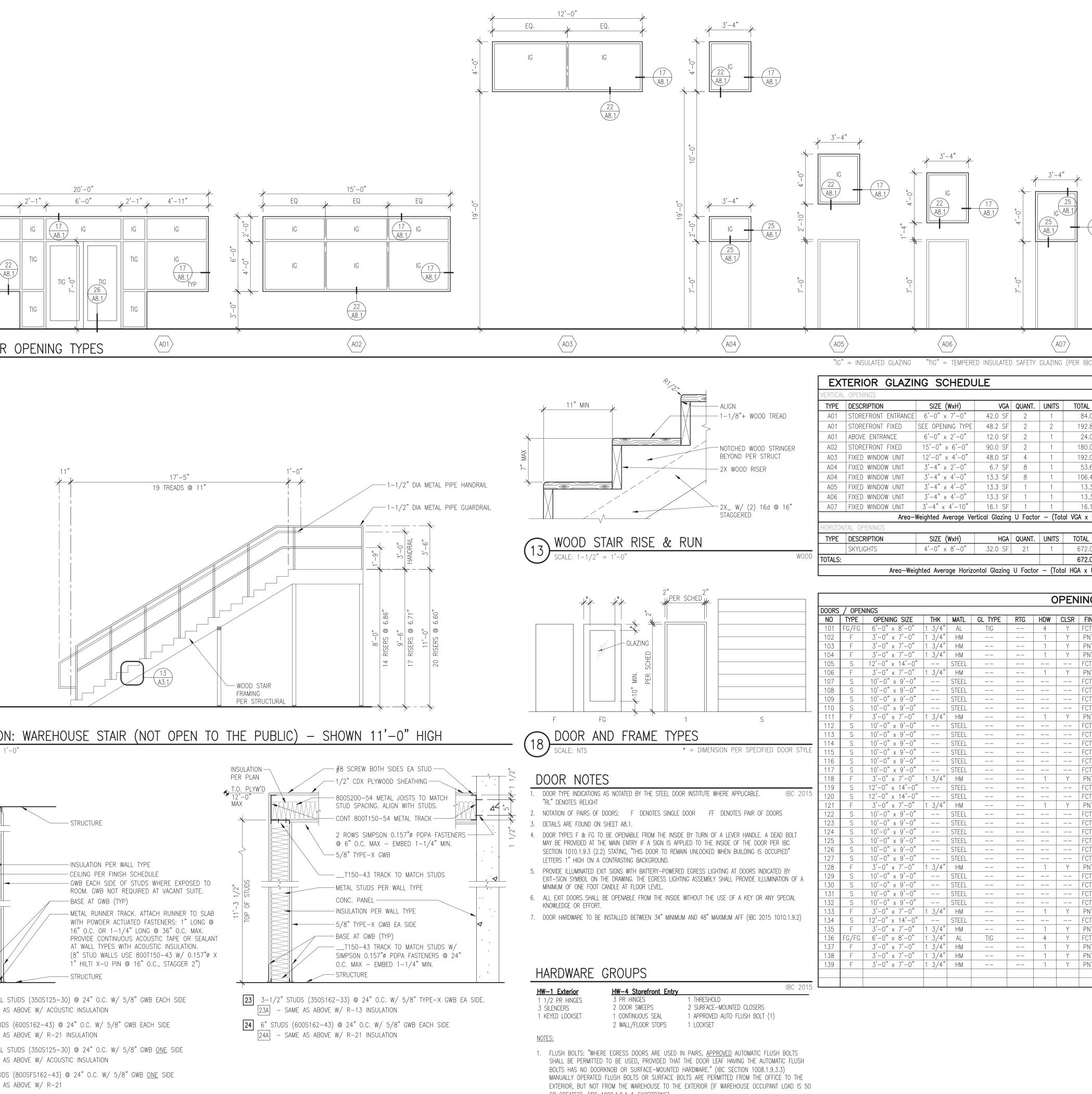
ROOF PLAN

DESIGNED BY: DRAWN BY: REVIEWED BY: 08 07 19 DATE: SHEET NO:

APPROVED BY: A2.3

PROJECT NO: 201613.03.003





PROVIDE HORIZ BRIDGING @ 48" OC @ STUD FLANGES WHERE GWB IS OMITTED WALL TYPES

2. MAIN STOREFRONT DOORS ARE PERMITTED TO BE EQUIPPED WITH KEY-OPERATED LOCKING DEVICES FROM THE EGRESS SIDE. SEE NOTE UNDER <u>DOOR NOTES</u>. (IBC SECTION 1008.1.9.3.2)

3. THE UNLATCHING OF ANY DOOR OR LEAF SHALL NOT REQUIRE MORE THAN ONE OPERATION. (IBC SECTION 1008.1.9.5)

4/15

25 A8.1	SAFETY CAP , 0, WX , 0	2'-6"	SAFE	2° X 1 1/ ETRACTIBLE TY POST 2°- - STEEL - SAFETY CAGE - 3/4″ø RU CAPABLE WITHSTAN A 300Ib TYP	3" 3" 3" 3" 4 4 4 5 5 10 10 10 10 10 10 10 10 10 10	N I <u>EW</u>	<text><text><section-header><section-header><section-header><section-header><section-header></section-header></section-header></section-header></section-header></section-header></text></text>
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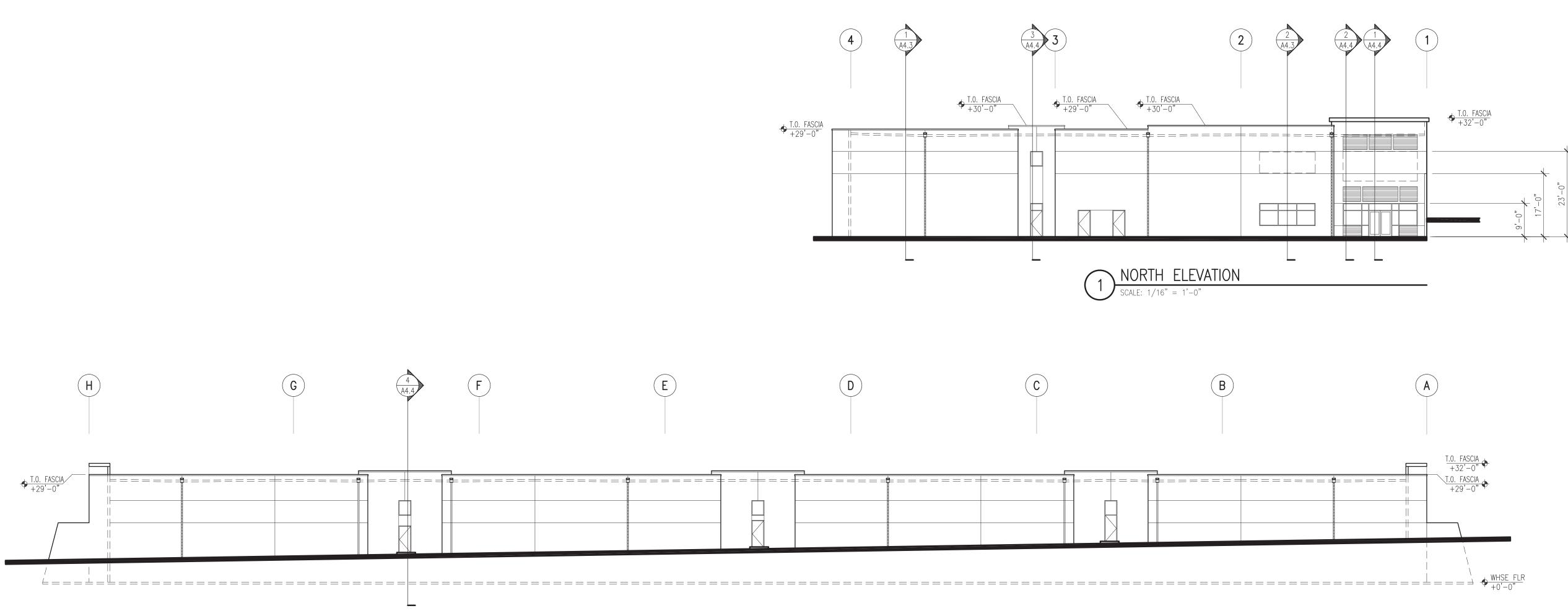
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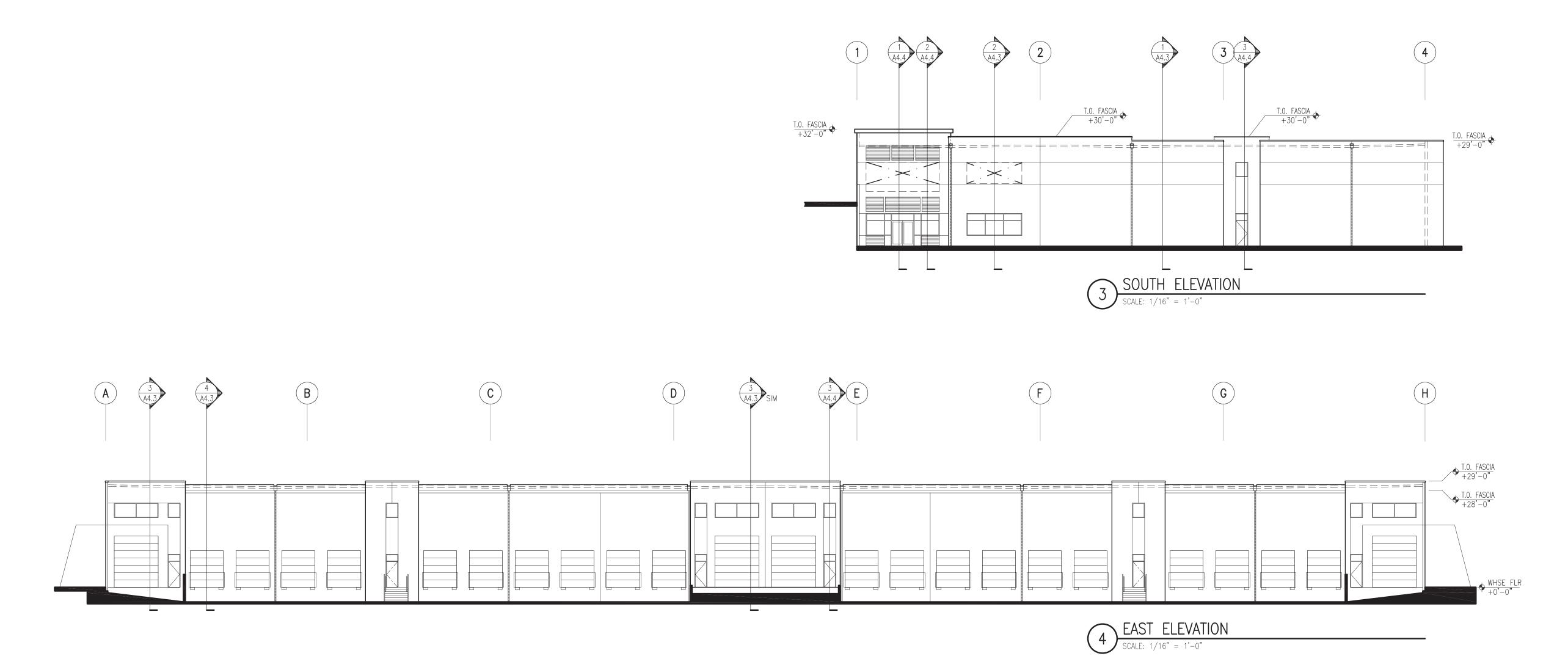
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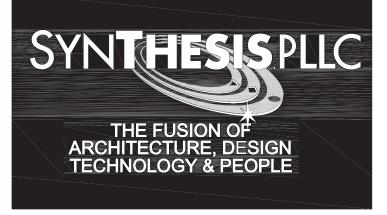
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A3.1





WEST ELEVATION (NEW) SCALE: 1/16" = 1'-0"



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PROJECT INFORMATION

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TITLE:

release for: BUILDING PERMIT EXTERIOR ELEVATIONS

DESIGNED BY: REVIEWED BY: DATE:

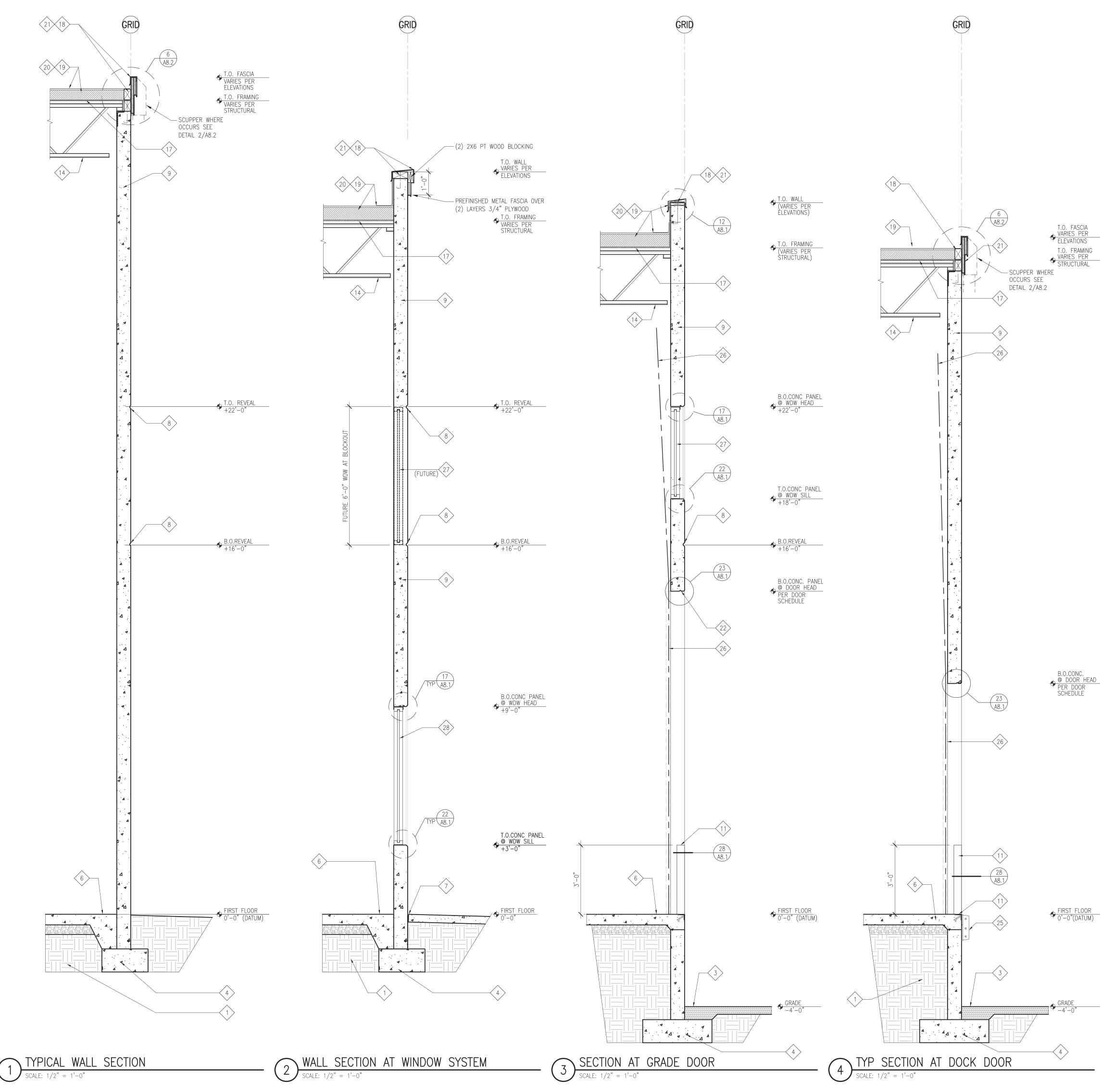
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PROJECT NO: 201613.03.003

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KEYNOTES - WALL SECTIONS

NOTE: NOT ALL KEYNOTES ARE USED $\langle 1 \rangle$ STRUCTURAL FILL

 $\langle 2 \rangle$ CURTAIN BOARD BEYOND

 $\langle 3 \rangle$ AC PAVING

 $\langle 4 \rangle$ conc footing, see struct dwgs for elevation CONC WALK OR LANDING OVER COMPACTED FILL PER SOILS REPORT CONC SLAB-ON-GRADE ON GRAVEL CAPILLARY BREAK ON STRUCTURAL FILL PER SOILS REPORT

7 > SEALANT (W/ BACKER ROD AS NEEDED)

HORIZONTAL REVEAL, SEE DETAIL 13/A8.1, TYP UON

 \langle 9 \rangle SITE CAST CONC WALL PANEL, PAINT

(11) STEEL ANGLE, SEE STRUCT DWGS.

(12) STEEL GIRDER, SEE STRUCTURAL FRAMING PLAN

STEEL BEAM, PAINT. SEE STRUCTURAL FRAMING PLAN

 $\langle 14 \rangle$ Steel joist, see structural framing plan

TUBE STEEL COLUMN OR BEAM, SEE STRUCTURAL DRAWINGS $\langle 16 \rangle$ FILTER FABRIC

(17) OSB WOOD ROOF DECK, SEE STRUCTURAL DWGS

 $\langle 18 \rangle$ wood nailer and blocking

1/2" PROTECTION BOARD ON RIGID INSULATION W/ VAPOR BARRIER & PERIMETER WD BLKG AS REQD SINGLE-PLY MEMBRANE ROOF ASSEMBLY, CLASS B, OR HIGHER

 $\langle 21 \rangle$ PREFINISHED METAL FASCIA

 $\langle 22 \rangle$ DRIP FORMED IN CONC PANEL

 $\langle 23 \rangle$ SURFACE REGLET

 $\langle 24 \rangle$ CRUSHED ROCK

 $\langle 25 \rangle$ DOCK BUMPER

 $\langle 26 \rangle$ SECTIONAL OVERHEAD DOOR TRACK

 $\langle 27 \rangle$ Aluminum window system

<28 ALUMINUM STOREFRONT SYSTEM

 $\langle 29 \rangle$ Foundation drain

 $\langle 30 \rangle$ GLAZING TYPE IG

 $\langle 31 \rangle$ GLAZING TYPE TIG

(34)

37

(32) STEEL CHANNELS, SEE STRUCTURAL DRAWINGS

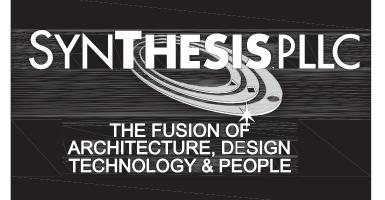
(33) GLAZING TYPE SPGL (SPANDREL GLASS).

WALL SCONCE LIGHT FIXTURE EA SIDE OF ENTRY DOOR.

WATERPROOFING MEMBRANE SUBSURFACE DRAINAGE

36 PATTERN USING CONCRETE FORM LINER

R-10 PERIMETER RIGID INSULATION WITH TAPERED TOP EDGE



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RELEASE FOR: BUILDING PERMIT TITLE:

WALL SECTIONS

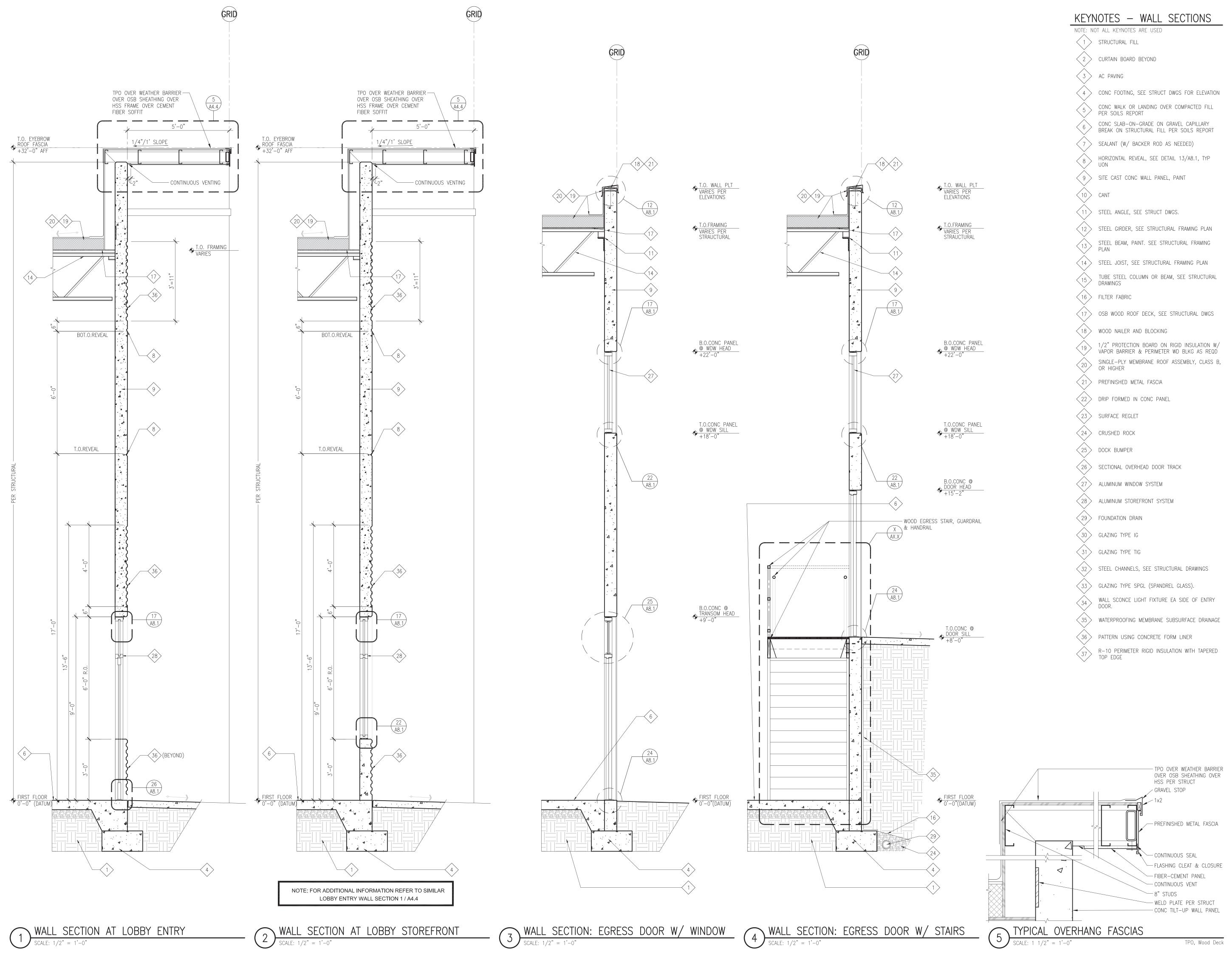
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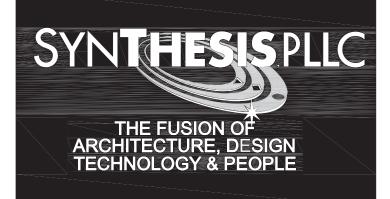
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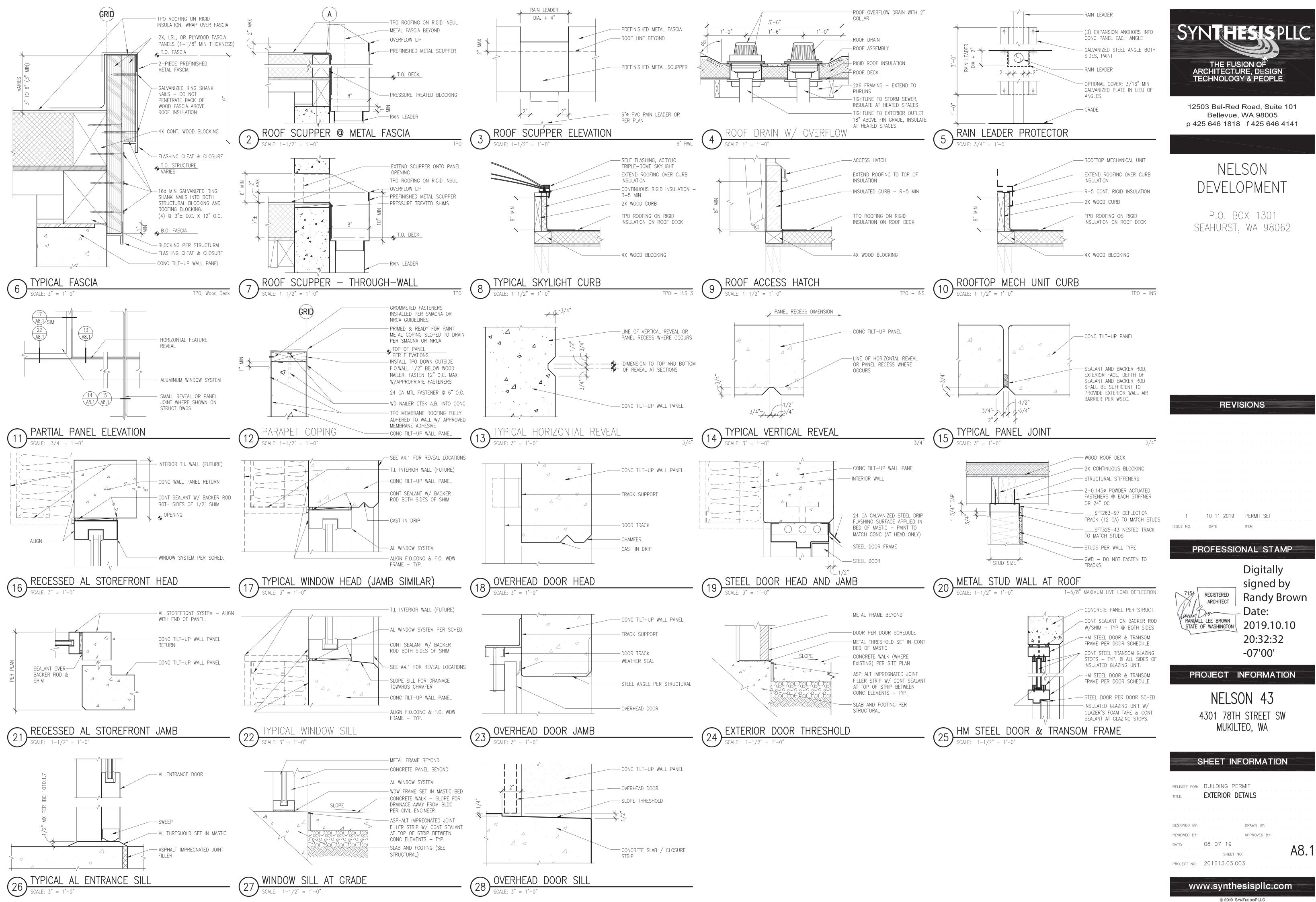
RELEASE FOR: BUILDING PERMIT WALL SECTIONS TITLE:

PROJECT NO: 201613.03.003

DESIGNED BY: DRAWN BY: APPROVED BY: REVIEWED BY: 08 07 19 DATE: SHEET NO:

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SHOP DRAWINGS ARE AN AID FOR FIELD PLACEMENT AND ARE SUPERSEDED BY THE STRUCTURAL DRAWINGS. IT SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO MAKE CERTAIN THAT ALL CONSTRUCTION IS IN FULL AGREEMENT WITH THE LATEST STRUCTURAL DRAWINGS.

SPECIAL CONDITIONS:

THE DRAWINGS INDICATE THE STRUCTURE IN ITS FINAL CONDITION. DURING CONSTRUCTION THE CONTRACTOR SHALL PROVIDE ADEQUATE SHORING, BRACING, AND GUYING IN ACCORDANCE WITH SOUND PRACTICE AND ALL NATIONAL, STATE, AND LOCAL CODES. CONTRACTOR TO COORDINATE ALL TRADES AND VERIFY DIMENSIONS IN FIELD. OBTAIN ARCHITECT'S APPROVAL PRIOR TO ALL FIELD CHANGES. SEE ARCHITECTURAL DRAWINGS FOR ALL FLOOR AND WALL OPENING DIMENSIONS AND LOCATIONS, FLOOR AND WALL FINISHES, ETC.

MECHANICAL, FIRE PROTECTION AND ELECTRICAL SYSTEMS:

T SHALL BE THE GENERAL CONTRACTOR AND/OR SUB-CONTRACTOR'S RESPONSIBILITY TO PROVIDE ALL NECESSARY ENGINEERING AND CONSTRUCTION DOCUMENTS FOR THEIR SCOPE OF WORK. THIS SHALL INCLUDE THE DESIGN OF THEIR SYSTEM AND ANY REVIEW AND MODIFICATIONS OF THE BASIC STRUCTURAL SYSTEM SHOWN ON THESE CONSTRUCTION DOCUMENTS AS WELL AS ANY ADDITIONAL STRUCTURAL SUPPORT REQUIRED BY THEIR SYSTEM.

STAIR

STAIRS TO BE DESIGNED BY OTHERS. SHOP DRAWINGS ALONG WITH STRUCTURAL CALCULATIONS STAMPED BY A LICENSED WASHINGTON STATE STRUCTURAL ENGINEER SHALL BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION. STAIR STRINGERS SHALL BE C12 X 20.7 MINIMUM. STAIR LANDING SUPPORT POSTS SHALL BE HSS 3 X 3 X 3/16 MINIMUM.

GENERAL NOTES CONTINUED: **PRECAST PANELS:**

PANEL LIFTING STRESSES ARE TO BE CHECKED BY THE CONTRACTOR AND HE SHALL PROVIDE REINFORCING STEEL AS REQUIRED FOR HIS METHOD OF HANDLING AND ERECTION OF PRECAST PANELS. USE STRONGBACKS AS REQUIRED AT EXCESSIVE PANEL OPENINGS. CONTINUOUS GROUT BETWEEN PANELS AND FOOTINGS TO BE "EMBECO" MASTER FLOW #713 BY MASTER BUILDERS, INC. OR APPROVED EQUAL. SEE DRAWINGS FOR ADDITIONAL NOTES **REGARDING PRECAST PANELS.**

STATEMENT OF SPECIAL INSPECTIONS:

SPECIAL INSPECTIONS SHALL BE PERFORMED AS OUTLINED IN THE STRUCTURAL NOTES. TESTING SHALL BE IN ACCORDANCE WITH THE SPECIAL INSPECTION SECTION OUTLINED ON THIS DRAWING SHEET. A SEPARATE STATEMENT OF SPECIAL INSPECTIONS AS REQUIRED PER IBC SECTION 1705 SHALL BE PROVIDED BY OTHERS FOR ALL MATERIALS, SYSTEMS, COMPONENTS AND WORK NOT INCLUDED IN THE STRUCTURAL DRAWINGS.

SPECIAL INSPECTION FOR SEISMIC RESISTANCE

SPECIAL INSPECTION FOR SEISMIC RESISTANCE IS REQUIRED FOR THE FOLLOWING COMPONENTS AND SYSTEMS:

1) ROOF TOP HVAC UNITS CONTAINING HAZARDOUS MATERIALS

2) WALL TIES 3) ROOF DIAPHRAGM

4) SHEAR WALLS

5) HOLD-DOWNS

6) CONCRETE WALL REINFORCING AND CONNECTIONS 7) FOUNDATION REINFORCING

CONTRACTORS RESPONSIBILITY (IBC SECTION 1704.4):

THE CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF THE MAIN WIND OR SEISMIC FORCE RESISTING SYSTEM. DESIGNATED SEISMIC SYSTEM, OR WIND/SEISMIC RESISTING COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. THE CONTRACTOR'S

- STATEMENT OF RESPONSIBILITY SHALL CONTAIN THE FOLLOWING: 1. ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN
- THE STATEMENT OF SPECIAL INSPECTIONS; 2. ACKNOWLEDGMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE
- WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL; 3. PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION, THE METHOD AND FREQUENCY OF REPORTING AND DISTRIBUTION OF THE REPORTS;
- 4. IDENTIFICATION AND QUALIFICATIONS OF THE PERSON(S) EXERCISING SUCH CONTROL AND THEIR POSITION(S) IN THE ORGANIZATION.

SPECIAL INSPECTIONS:

SPECIAL INSPECTIONS PER IBC SECTION 1705. A COPY OF ALL INSPECTION REPORTS AND TEST RESULTS FOR ALL REQUIRED INSPECTIONS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT, SHUTLER CONSULTING ENGINEERS, THE OWNER AND THE ARCHITECT BY THE TESTING AGENCY FOR REVIEW. THE TESTING AGENCY SHALL BE AN INDEPENDENT TESTING AGENCY APPROVED BY THE BUILDING DEPARTMENT. THE FOLLOWING INSPECTIONS SHALL BE PROVIDED AS A MINIMUM; ADDITIONAL INSPECTIONS AS REQUIRED BY THE BUILDING DEPARTMENT SHALL ALSO BE PERFORMED. JOB SITE VISITS BY THE ENGINEER DO NOT CONSTITUTE AN OFFICIAL INSPECTION. SPECIAL INSPECTION OF THE PANELIZED ROOF STIFFENER HANGERS SHALL BE PER THE DETAIL ON SHEET S-3.0.

SEE SPECIAL INSPECTION FREQUENCY TABLE ON THIS SHEET FOR BOTH ON SITE CONSTRUCTION AND OFF SITE FABRICATION .

SPRINKLER SUPPORT NOTES:

1A. ALL SPRINKLER PIPE LARGER THAN 3" NOMINAL DIAMETER IS TO BE CONSIDERED A "MAIN" FOR THE PURPOSE OF THESE NOTES.

B. THE BIDS FOR BOTH THE SPRINKLER AND JOISTS ARE TO INCLUDE ALL HANGERS, BRACES, ETC., AND ACCOUNT FOR THE LOADS INDICATED.

2A. ALL MAINS RUNNING PERPENDICULAR TO THE JOISTS ARE TO BE VERTICALLY

B. EACH JOIST THAT IS CROSSED BY A MAIN IS TO BE DESIGNED FOR THE VERTICAL SUPPORT LOAD BASED ON SUPPORTS AT EVERY OTHER JOIST.

5. ALL MAINS RUNNING PARALLEL TO THE JOISTS MUST BE: A. PLACED MID-WAY BETWEEN TWO JOISTS AND BE TRAPEZED SO EACH JOIST SUPPORTS HALF THE LOAD

B. SUPPORTED AT 10'-0" ON CENTER MAXIMUM.

4. USE THE FOLLOWING WEIGHTS FOR SPRINKLER VERTICAL SUPPORT DESIGN: WT PER FOOT HANGER VERTICAL LOAD EXAMPLE. PIPE SIZE

8"	51#	GIVEN: 8" DIAMETER MAIN
		SUPPORTS AT 5' ON CENTER
6"	32#	FIND: HANGER DESIGN LOAD
5"	24#	SOLN: 51#/FT X 10' = 510#
4"	17#	ADDN'L PER NFPA #13 = 250#
3-1/2"	14#	TOTAL SUPPORT DESIGN LOAD = 760#/HG

ALL HANGERS TO BE DESIGNED, SUPPLIED AND INSTALLED BY THE SPRINKLER SUPPLIER.

5. SPRINKLER LATERAL SWAY BRACING (AT 90 DEGREES TO MAIN) TO BE:

A. SPACED A MAXIMUM OF TWICE THE VERTICAL SUPPORT SPACING.

B. BRACED FROM THE MAIN TO A ROOF STRUCTURAL MEMBER WITH CONNECTIONS DESIGNED AND INSTALLED BY THE SPRINKLER SUPPLIER.

C. DESIGNED FOR A LATERAL LOAD OF THE WEIGHT PER FOOT OF HALF THE WATER FILLED PIPE WEIGHT TIMES THE SWAY BRACE SPACING.

6. THE SPRINKLER DESIGNER IS TO PROVIDE THE CONTRACTOR, ARCHITECT AND JOIST MANUFACTURER WITH DETAILS OF HANGER AND SWAY BRACE ATTACHMENTS WITH DESIGN LOADS, SO THESE LOADS CAN BE DESIGNED INTO THE JOISTS, ETC.

7. THE ROOF JOIST DESIGNER TO PROVIDE AND INSTALL ADDITIONAL BRACES TO TAKE THE SWAY BRACE LOAD FROM THE SWAY BRACE INTO THE ROOF DIAPHRAGM.

DEFERRED SUBMITTALS

THE FOLLOWING ITEMS ARE DEFERRED STRUCTURAL COMPONENTS SUBMITTALS. REFER TO ARCHITECTURAL, MECHANICAL ELECTRICAL AND CIVIL DRAWINGS FOR ADDITIONAL DEFERRED SUBMITTAL COMPONENTS.

1. STEEL JOIST AND GIRDERS

2. PRE-ENGINEERED STEEL STAIRS (SEE STAIR NOTES) 3. MECHANICAL AND ELECTRICAL COMPONENT CONNECTIONS & SUPPORTS

DOCUMENTS FOR DEFERRED SUBMITTALS SHALL BE SUBMITTED TO SHUTLER CONSULTING ENGINEERS, INC. WHO SHALL REVIEW THEM FOR GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTALS SHALL THEN SUBMITTED TO THE BUILDING DEPARTMENT BY THE GENERAL CONTRACTOR. DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL APPROVED BY THE BUILDING OFFICIAL.

SHOP DRAWINGS:

SHOP DRAWINGS FOR THE FOLLOWING ITEMS SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS CONSISTING OF REPRODUCTIONS OR COPIES OF ANY PORTIONS OF THE STRUCTURAL DRAWINGS WILL NOT BE ACCEPTED AS SHOP DRAWINGS NOR REVIEWED BY THE STRUCTURAL ENGINEER AS SUCH.

1) REINFORCING STEEL

a)structural concrete elements i.e. beams, walls, col, ftgs, etc.

2) STRUCTURAL STEEL ITEMS 3) STEEL JOISTS & GIRDERS

THE ENGINEER OF RECORD WILL REVIEW SHOP DRAWINGS FOR DESIGN INTENT ONLY. VERIFICATION OF DIMENSIONS AND QUANTITIES ARE THE RESPONSIBILITY OF THE CONTRACTOR AND ARE NOT GUARANTEED BY THE ENGINEER OF RECORD. DRAWINGS FOR COMPONENTS DESIGNED PRIMARILY BY THE MANUFACTURER SHALL BEAR THE STAMP OF A LICENSED WASHINGTON STATE STRUCTURAL ENGINEER AND BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR A CURSORY REVIEW FOR COMPLIANCE WITH THE INTENT OF THE STRUCTURAL DRAWINGS AND FOR LOADS IMPOSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. SUBMISSIONS SHALL INCLUDE A REPRODUCIBLE AND TWO COPIES; REPRODUCIBLE WILL BE REVIEWED AND RETURNED. SHOP DRAWINGS MUST BE REVIEWED AND STAMPED BY THE CONTRACTOR PRIOR TO REVIEW BY THE ENGINEER.

REINFORCING STEEL CONTINUED:

REBAR FOR TILT-UP PANELS SHALL BE A706. A615 REBAR IS ACCEPTABLE IF MILL TESTS SHOW: 1) Fv DOES NOT EXCEED 60,000 psi BY MORE THAN 18,000 psi, AND 2)THE RATIO OF ACTUAL TENSILE STRENGTH TO ACTUAL YIELD STRENGTH IS NOT LESS THAN 1.25.

	MINIMUM LAP	P SPLICE LENGTHS FOR CONCRETE
BAR SIZE	<u>fc=3500</u>	<u>fc=5000</u>
#4	26"	23"
#5	33"	29"
#6	40"	34"
#7	58"	49"
#8	66"	56"
#9	74"	63"
#10	84"	70"
#11	93"	78"

THE FOLLOWING MINIMUM COVER SHALL BE PROVIDED FOR REINFORCEMENT (UNLESS INDICATED OTHERWISE ON DRAWINGS):

A)	CONCRETE CAST AGAINST & PERMANENTLY EXPOSED TO
	EARTH3" EARTH FACE
B)	CONCRETE EXPOSED TO EARTH OR WEATHER (CAST IN FORMS)
	#6 THROUGH #18 BARS2"
	#5 BARS & SMALLER1-1/2"
C)	CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND
	SLABS, WALLS & JOIST - #11 BAR & SMALLER3/4"
	BEAMS & COLUMNS1-1/2"
D)	PRECAST PANELS
	#8 BARS & SMALLER1" EXTERIOR FACE
	#9 BARS & LARGER

(SEE ITEM 'C' ABOVE FOR INTERIOR FACE.)

WELDED REBAR:

WELDED REBAR TO BE GRADE 60 CONFORMING TO ASTM A706. (PREHEAT ALL GRADE 60 REBAR PRIOR TO WELDING.) USE FRESH E80XX LOW HYDOGEN ELECTRODES TO WELD GRADE 60 REINFORCING; CONFORM TO PROCEDURES OF AWS D1.4, RECOMMENDED PRACTICES FOR WELDING REINFORCING STEEL. SEE SPECIAL INSPECTION PROGRAM. REINFORCING TO BE WELDED SHALL NOT HAVE CARBON CONTENT IN EXCESS OF .35%. ALL WELDS TO BE BY WABO CERTIFIED WELDERS.

STRUCTURAL STEEL

RECTANGULAR AND SQUARE HSS SECTIONS SHALL CONFORM TO ASTM A500, GRADE C (Fy = 50,000 psi). ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 (Fy = 36,000 psi) OR ASTM A992 GRADE 50 (Fy = 50,000 psi). APPLY PRIMER COATS PER ARCHITECTURAL SPECIFICATIONS. WELDS NOT SPECIFIED SHALL BE 3/16" CONTINUOUS FILLET MINIMUM. ALL WELDS TO BE IN ACCORDANCE WITH AWS D1.1 AND D1.8 AND BY WABO CERTIFIED WELDERS. USE FRESH E70XX LOW HYDROGEN ELECTRODES. ALL STRUCTURAL WELDS SHOULD BE CONSIDERED PART OF THE SEISMIC FORCE RESISTING SYSTEM AND SHALL BE MADE WITH FILLER METAL PRODUCING WELDS WITH A MINIMUM CHARPY V-NOTCH TOUGHNESS OF 20 FT-LBS AT 0 DEGREES-F AS DETERMINED BY THE APPLICABLE AWS A5 CLASSIFICATION TEST METHOD. DEMAND CRITICAL WELDS, AS SHOWN ON THE STRUCTURAL DRAWINGS, SHALL MEET THE REQUIREMENTS OF D1.8. THE CONTRACTOR SHALL SUBMIT TO THE BUILDING OFFICIAL A WELDING PROCEEDURE SPECIFICATION THAT VERIFIES THESE REQUIREMENTS.

MISCELLANEOUS STEEL CONNECTORS TO BE SIMPSON, OR APPROVED EQUAL. NAIL ALL HOLES WITH NAILS AS SPECIFIED BY MANUFACTURER UNLESS SHOWN OTHERWISE ON DRAWINGS. ALL NAILS ATTACHING TO PRESSURE TREATED WOOD MEMBERS SHALL BE HOT DIPPED GALVANIZED.

MACHINE BOLTS TO BE A307.

HIGH STRENGTH BOLTS FOR STEEL TO STEEL CONNECTIONS SHALL BE ASTM A325N OR ASTM A490, INSTALLATION AND SPECIAL INSPECTION PER AISC SPECIFICATIONS. ALL A325N OR A490 BOLTS SHALL BE PRETENSIONED IN STANDARD BOLT HOLES UNLESS NOTED OTHERWISE ON PLANS. MINIMUM BOLT PRETENSION PER AISC 16.1 TABLE J3.1 UNLESS NOTED OTHERWISE. ACCEPTABLE PRETENSIONING METHODS ARE TURN-OF-NUT. CALIBRATED WRENCH, TWIST-OFF-TYPE (ASTM F1852), OR DIRECT-TENSION INDICATOR (ASTM F959) PER AISC SPECIFICATIONS.

STEEL JOISTS AND JOIST GIRDERS:

STEEL JOISTS ARE TO BE DESIGNED AND FABRICATED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION SPECIFICATIONS, 14TH EDITION AND STANDARD SPECIFICATIONS FOR STEEL JOISTS AS ADOPTED BY THE STEEL JOIST INSTITUTE AND PROVIDE A CERTIFICATION OF COMPLIANCE AT THE COMPLETION OF FABRICATION. JOIST MANUFACTURER TO PROVIDE ALL BRIDGING AND BLOCKING, BOTH PERMANENT AND ERECTION. SHOP DRAWINGS AND DESIGN CALCULATIONS, STAMPED BY A LICENSED WASHINGTON STATE STRUCTURAL ENGINEER, ARE TO BE SUBMITTED FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS WILL NOT BE REVIEWED UNLESS THEY BEAR THE STAMP OF A LICENSED WASHINGTON STATE STRUCTURAL ENGINEER AND CALCULATIONS STAMPED BY THE SAME ENGINEER ARE SUBMITTED ALONG WITH THE SHOP DRAWINGS. DESIGN CRITERIA SHALL MEET OR EXCEED THE FOLLOWING:

ROOF IOIST LOADING -- TOP CHORD

ROOF JOISTLOADING 7	TOP CHORD	25 PSF L.L. PLUS
		9 PSF D.L.
B	OTTOM CHORD	6 PSF D.L.
ROOF JOISTDEFLECTION	LIVE LOAD	L/360
	TOTAL LOAD	L/240

STEEL JOIST SUPPLIERS NOTE:

THE JOIST CONFIGURATIONS. INCLUDING DEPTHS AND SPACING. SHOWN ON THE DRAWINGS INDICATE THE DESIRED JOIST CONFIGURATION AND ARE TO BE COMPLIED WITH WHEREVER POSSIBLE. IF A JOIST MANUFACTURER IS UNABLE TO MEET THE LOAD REQUIREMENTS SPECIFIED WITH THE JOIST CONFIGURATION INDICATED, HE OR SHE IS TO SUBMIT WRITTEN NOTICE TO THAT AFFECT TO THE ARCHITECT PRIOR TO SUBMITTING A COST PROPOSAL OR BID.

IF A DIFFERENT SYSTEM IS PROPOSED THAT REQUIRES REVISIONS TO PRESENT STRUCTURAL FRAMING OR DETAILS, SUCH SYSTEM SHALL BE CONSIDERED SUBJECT TO THE APPROVAL OF THE OWNER, ARCHITECT AND ENGINEER.

WHERE IT IS NECESSARY TO BUTT OPPOSITE JOISTS OVER A NARROW SUPPORT WITH BEARING LENGTH LESS THAN MINIMUM REQUIREMENT, SPECIAL END CONDITIONS (SHORT BEARING LENGTH) SHALL BE DESIGNED BY THE JOIST MANUFACTURER PER SJI SPECIFICATION SECTION 104.4(b).

IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND JOIST MANUFACTURER TO VERIFY THE WEIGHT AND LOCATIONS OF ALL MECHANICAL EQUIPMENT PRIOR TO SUBMITTING SHOP DRAWINGS. IT SHALL BE NOTED IN THE JOIST MANUFACTURER'S BID WHETHER OR NOT AN ALLOWANCE HAS BEEN MADE FOR MECHANICAL UNITS.

FRAMING LUMBER:	R	REFERENCE DESIGN VALUES
STEEL JOIST PURLIN NAILERS	D.F. #2	Fb = 900 psi
2x, 3x, 4x MEMBERS	D.F. #2	Fb = 900 psi
6x MEMBERS	D.F. #2	Fb = 875 psi
4x POSTS		1
6x POSTS		

LUMBER NOT NOTED SHALL BE D.F. #2 OR BETTER.

ALL GRADES SHALL CONFORM TO WWPA WESTERN LUMBER GRADING RULES -- 2011 EDITION. ALL BOLTS HEADS AND NUTS BEARING AGAINST WOOD SHALL BE PROVIDED WITH STANDARD CUT WASHERS EXCEPT AS NOTED FOR ANCHOR BOLTS UNDER STRUCTURAL STEEL SECTION. ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED. MAXIMUM MOISTURE CONTENT 19% AT INSTALLATION FOR ALL LUMBER.

WOOD SHEATHING

ROOF SHEATHING......15/32" OR 19/32" OSB OR CDX PLYWOOD STRUCTURAL I RATED PANELS WITH STRENGTH AXIS PARALLEL TO SUPPORTS W/EXTERIOR GLUE.....INDEX 32/16

ALL PLYWOOD SHALL CONFORM TO U.S. PRODUCT STANDARD P.S. 1-09. ALL O.S.B. SHALL CONFORM TO U.S. PRODUCT STANDARD P.S. 2-10. NAILING SHALL BE AS INDICATED ON PLAN. HIGH SHEAR DIAPHRAGMS SHALL HAVE SPECIAL INSPECTIONS AS OUTLINED BELOW. CONTRACTOR IS TO CALL THE ENGINEER TO SCHEDULE SITE VISIT PRIOR TO COVERING ROOF SHEATHING.

(GENERAL NOTES CONTINUED ABOVE LEFT)

GENERAL NOTES

(SEE SLAB ON GRADE NOTES)

Pf = 25 PSF (WITHOUT DRIFT)

REDUCIBLE

NO

CODE: INTERNATIONAL BUILDING CODE, 2015 EDITION

BUILDING RISK CATEGORY = II

LIVE LOADS: UNIFORM LOADS:

LOCATION SLAB ON GRADE ROOF

SNOW LOAD DATA Pg = 20 PSFCe = 0.9Ct = 1.0

 $I_{s} = 1.0$

WIND DESIGN INFORMATION: WIND LOADS ON THE MAIN WIND-FORCE RESISTING SYSTEM (MWFRS) WAS DETERMINED

USING THE ENVELOPE PROCEDURE WIND SPEED (BASED ON TABLE 1609.3.1 OF IBC 2015): $V_{ult} = 110 \text{ MPH}$ $V_{asd} = 85 \text{ MPH}$ EXPOSURE "B" $\lambda = 1.00$ K_{zt}=1.00

LIVE LOAD

Pd = N/A

W = N/A

25 PSF (SNOW)

SEISMIC DESIGN INFORMATION:

SEISMIC FORCE RESISTING SYSTEM IS A BEARING WALL SYSTEM WITH SPECIAL REINFORCED CONCRETE SHEAR WALLS. THE STRUCTURE WAS ANALYZED USING THE EQUIVALENT LATERAL FORCE PROCEDURE

LATLICALIONCLIN	OCLDURL.
R = 5.0	$\Omega_{\rm O} = 2.0$
$S_{S} = 1.453$	$S_{DS} = 0.9$
$S_1 = 0.565$	$S_{D1} = 0.50$
$C_{\rm S} = 0.194$	

 $I_{\rm E} = 1.0$ SITE SOIL CLASS = DSEISMIC DESIGN CATEGORY = D

BASE SHEAR, V = 761 KIPS (ULTIMATE)

FOUNDATIONS

FOUNDATION DESIGN PER GEOTECHNICAL REPORT #ES-6384 DATED DECEMBER 27TH, 2018, BY EARTH SOLUTIONS NW. LLC. ALL FOUNDATION WORK PER THIS REPORT. DESIGN CRITERIA INDICATED IN THE GEOTECHNICAL REPORT ARE AS FOLLOWS. ALLOWA 500 PSF

ALLOWABLE SOIL BEARING CAPACITY	.2500 PSF
PASSIVE SOIL RESISTANCE	350 PCF
ACTIVE SOIL PRESSURE	35 PCF
COEFFICIENT OF FRICTION	0.40
SEISMIC EARTH PRESSURE	6Н

ALL EXTERIOR FOOTINGS TO BE A MINIMUM OF 18 INCHES BELOW LOWEST ADJACENT GRADE. SOILS ENGINEER TO INSPECT AND APPROVE FOUNDATION EXCAVATIONS PRIOR TO POURING. ALL FOOTINGS AND SLABS SHALL BEAR ON COMPETENT NATIVE SOIL. RECOMPACTED NATIVE SOIL, OR NEW STRUCTURAL FILL. SEE THE SOILS REPORT FOR SPECIFIC FILL REQUIREMENTS, FILL PLACEMENT REQUIREMENTS, PRELOAD REQUIREMENTS AND ADDITIONAL INFORMATION.

SLABS ON GRADE

THE SLAB ON GRADE FOR THIS PROJECT IS TYPICAL OF OTHER BUILDINGS WITH SIMILAR FLOOR LOADING AND SOIL CONDITIONS CONSTRUCTED IN THIS AREA. THE SLAB HAS NOT BEEN DESIGNED FOR ANY SPECIFIC LIVE LOAD AND HAS BEEN DETAILED TO MEET LOCAL INDUSTRY STANDARDS FOR SIMILAR BUILDINGS. NO CONSTRUCTION LOADS HAVE BEEN INCLUDED IN THE DESIGN OF THE FLOOR SLAB.

THE CONTRACTOR SHALL REVIEW WITH THE SOILS ENGINEER THE CONSTRUCTION LOADING OF THE SLAB AND SOILS BELOW. HE SHALL TAKE THE NECESSARY MEASURES TO INSURE THAT THE SLAB AND SOILS BELOW WILL NOT BE AFFECTED OR DAMAGED BY THE CONSTRUCTION LOADING. THE CONTRACTOR SHALL ADD ADDITIONAL CONCRETE, REINFORCING AND UPGRADE JOINT DETAILING AS REQUIRED FOR HIS LOADING.

CONTROL JOINTS SHALL BE SOFF CUT JOINTS PER THE DETAILS ON THE STRUCTURAL DRAWINGS. JOINTS SHALL BE CUT 0 TO 2 HOURS AFTER FINAL FINISH AT EACH JOINT LOCATION (AS SOON AS THE SLAB WILL SUPPORT THE SOFF CUT MACHINE AND OPERATOR). COMPLY WITH THE SOFF-CUT INTERNATIONAL, INC. INSTRUCTIONS FOR THE SOFF CUT SYSTEM. ALL JOINTS SHALL BE FILLED WITH MM-80 AS MANUFACTURED BY METZER MCGUIRE OR APPROVED EQUAL. THE JOINT FILLER SHALL BE INSTALLED IN CONFORMANCE WITH THE MANUFACTURER'S SPECIFICATIONS. INSTALL THE JOINT FILLER AS LONG AFTER THE SLAB HAS BEEN POURED AS POSSIBLE, BUT NOT LESS THAN 90 DAY AFTER THE SLAB HAS BEEN POURED. THE JOINT SHALL BE CLEAN AND SOUND, AND FREE OF ALL OIL, DIRT, DEBRIS, PAINT AND ANY OTHER MATERIAL THAT MAY BE A BOND BREAKER. THE CONCRETE CURING COMPOUND MUST BE REMOVED PRIOR TO INSTALLING THE JOINT FILLER AND/OR SURFACE SEALER.

CONCRETE:

ALL CONCRETE SHALL BE STONE-AGGREGATE CONCRETE HAVING A UNIT WEIGHT OF APPROXIMATELY 145 POUNDS PER CUBIC FOOT. 28 DAY COMPRESSIVE STRENGTH SHALL BE AS FOLLOWS:

CONSTRUCTION	$\underline{\mathbf{f}}_{\mathrm{C}}$	EXPOSURE	MAX W/C	AIR
	-	CLASS	RATIO	CONTENT
PRECAST WALL PANELS	5,000 PSI	F1, S0, W0, C1	0.55	5%
FOOTINGS	3,500 PSI	F1, S0, W0, C1	0.55	5%
PRECAST RETAINING WALLS	3,500 PSI	F1, S0, W0, C1	0.55	5%
INTERIOR SLAB ON GRADE	3,500 PSI	F0, S0, W0, C0	N/A	N/A

DESIGN MIX FOR SLABS ON GRADE SHALL PROVIDE A MIX WITH A MAXIMUM SHRINKAGE OF 0.03% AT 42 DAYS.

CONCRETE IN ALL EXTERIOR SLABS TO BE AIR ENTRAINED 6% PLUS OR MINUS 1%.

CONCRETE SUBMITTALS SHALL CONFORM TO ACI 318 CHAPTER 26. MIXING AND PLACING OF ALL CONCRETE SHALL BE IN ACCORDANCE WITH THE IBC AND ACI CODE 318. PROPORTION OF AGGREGATE TO CEMENT SHALL BE AS SUCH TO PRODUCE A DENSE, WORKABLE MIX, WITH A MAXIMUM SLUMP OF 5 INCHES, WHICH CAN BE PLACED WITHOUT SEGREGATION OR EXCESS FREE SURFACE WATER. 3/4" CHAMFER ON ALL EXPOSED CONCRETE EDGES UNLESS INDICATED OTHERWISE ON ARCHITECTURAL DRAWINGS. GROUT FOR COLUMN BASES SHALL BE "HI-FLOW GROUT" AS MANUFACTURED BY THE EUCLID CHEMICAL CO. OR APPROVED EQUAL.

ANCHORAGE TO CONCRETE:

EXPANSION BOLTS INTO CONCRETE SHALL BE SIMPSON STRONG-BOLT **2** AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. SPECIAL INSPECTION AND INSTALLATION PER ICC-ES REPORT ESR-3037.

CONCRETE SCREW ANCHORS SHALL BE SIMPSON TITEN HD SCREW ANCHORS AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. SPECIAL INSPECTION AND INSTALLATION PER ICC-ES REPORT ESR-2713.

EPOXY FOR FASTENING ANCHOR BOLTS AND REBAR INTO EXISTING CONCRETE TO BE SIMPSON SET-3G EPOXY ADHESIVE AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. SPECIAL INSPECTION AND INSTALLATION PER ICC-ES REPORT ESR-4057.

ANCHOR RODS INTO CONCRETE SHALL BE GRADE 36 MANUFACTURED AND INSTALLED PER ASTM F1554. ALL ANCHOR RODS/BOLTS ATTACHING PRESSURE TREATED WOOD PLATES TO CONCRETE SHALL BE HOT DIPPED GALVANIZED. ALL ANCHOR BOLTS FOR SILL PLATES SHALL BE PROVIDED WITH 3" X 3" X 0.229" THICK (MIN) PLATE WASHERS.

ANCHORAGE SUBSTITUTIONS MUST BE APPROVED BY THE ENGINEER. ANCHORAGE SUBSTITUTION REQUESTS REQUIRE ADDITIONAL ENGINEERING SERVICES.

REINFORCING STEEL:

ALL REINFORCEMENT SHALL CONFORM TO ASTM A615. (SEE BELOW FOR WELDED REBAR). ALL REINFORCING SHALL BE GRADE 60 (Fy = 60,000 psi; Fs = 32,000 psi). LAP CONTINUOUS REINFORCING BARS IN CONCRETE AS INDICATED BELOW. CORNER BARS (1'-7" BEND) WILL BE PROVIDED FOR ALL HORIZONTAL REINFORCEMENT. DETAIL STEEL IN ACCORDANCE WITH THE ACI MANUAL OF STANDARD PRACTICE OF DETAILING REINFORCED CONCRETE STRUCTURES. WELDED HEAD STUDS SHALL COMPLY WITH AWS D1. AND STUD MATERIAI SHALL COMPLY WITH ASTM A29 (Fy=65,000 psi).

(**REINFORCING STEEL** CONTINUED ABOVE LEFT)



12503 Bel-Red Road, Suite 101 Bellevue, WA 98005 p 425 646 1818 f 425 646 4141

SHUTLER CONSULTING ENGINEERS Inc 12503 Bel-Red Road, Suite 100

Bellevue, Washington 98005 (425)450-4075 FAX: (425)450-4076

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REVISIONS

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PROJECT INFORMATION

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PROJECT NO: 18-40

1 OF 13

STRUCTUR	AL DRAWING ABBREVIATIONS:	
A.B.	ANCHOR BOLT	LBS/#
ADJ.	ADJACENT	LF
A.F.F.	ABOVE FINISHED FLOOR	L.L.
ALT. APPROX.	ALTERNATE APPROXIMATELY	LLH LLV
ARCH.		LLV LSH
BM	BEAM	LSL
BLDG	BUILDING	LVL
BLKG	BLOCKING	M.B.
BRC	BRACING	M.B.S.
BRG BTWN	BEARING BETWEEN	MAX. MECH.
BOT.	BOTTOM	MEZZ.
СА	CAMBER	MFR
C.A. C.I.P.	CENTER-TO-CENTER	MTL
C.I.P.	CAST-IN-PLACE	MIN.
C.J.	CONSTRUCTION JOINT CENTERLINE	MISC. N.I.C.
¢_ CLG	CEILING	(N)
CLR	CLEAR	NO.
CMU	CONCRETE MASONRY UNIT	NOM.
COL.	COLUMN	N.S.
CONC.	CONCRETE	NTS
CONN. CONST.	CONNECTION CONSTRUCTION	o.c. O.D.
CONT.	CONTINUOUS	0.F.
CONTR.	CONTRACTOR	OPNG
COORD.	COORDINATE	OPP.
C.P.	COMPLETE PENETRATION	OSB
C.S. d	CLOSURE STRIP JOINT PENNY SIZE	OVS P
DBL	DOUBLE	P.A.
DET.	DETAIL	P.C.
DF	DOUGLAS FIR	PCF
DIA./Ø DIM.	DIAMETER	PEN.
DIM. D.L.	DIMENSION DEAD LOAD	₽ P.L.
D.C.	DITTO	F.L. Plywd
DN DN	DOWN	PNL
D.S.	DOWNSPOUT	PSF
DWG	DRAWING	PSI
DWL (E)	DOWEL EXISTING	PSL PT
EA.	EACH	P.T.
EL./ELEV.	ELEVATION	R.D.
EQ.	EQUAL	R.O.
EQUIP.	EQUIPMENT	REINF.
E.A. E.S.	EACH FACE EACH SIDE	REQ'D REV.
E.W.	EACH WAY	RTN
EXP.	EXPANSION	SCHED.
EXT.	EXTERIOR	SECT.
F.B.	FLUSH BEAM	SHT.
F.D. FDN	FOOTING DRAIN FOUNDATION	SHTHG SIM.
FIN.	FINISH	S.J.
FLG	FLANGE	S.O.G.
FLR	FLOOR	SP.
F.O.C. F.O.F.	FACE OF CONCRETE FACE OF FINISH	SPEC. SQ.
г.0.г. F.O.S.	FACE OF FINISH	SQ. SSH
F.P.	FULL PENETRATION	STD
F.S.	FAR SIDE	STIFF.
FT	FOOT	STL
FTG GA.	FOOTING GAGE	STRUCT. SYM
GA. GALV.	GAGE GALVANIZED	T&B
G.L.	GLULAM	T&G
GLB	GLULAM BEAM	THK
GR.	GRADE	TJI
GWB HD	GYPSUM WALL BOARD HOLDOWN	T.O.C. T.O.F.
HDR	HEADER	T.O.J.
HORIZ.	HORIZONTAL	T.O.W.
HSS	HOLLOW STRUCTURAL STEEL	U.N.O.
HT	HEIGHT	VERT.
IBC I.D.	INTERNATIONAL BUILDING CODE INSIDE DIAMETER	w/ WF
1.D. I.F.	INSIDE DIAMETER INSIDE FACE	w/o
IN.	INCH	WD
INFO.	INFORMATION	W.H.S.
INT.	INTERIOR	WP
JST K	JOIST KIP (1000 lbs)	WT W.W.F.
KSI	KIPS PER SQUARE INCH	**.**. .
KSF	KIPS PER SQUARE FOOT	

BS/#	
	LINEAR FOOT LIVE LOAD
	LONG LEG HORIZONTAL
LV	LONG LEG VERTICAL
SH	LONG SLOTTED HOLE
SL	TIMBERSTRAND MICROLLAM
	MACHINE BOLT
	METAL BUILDING SUPPLIER
	MAXIMUM
IECH.	MECHANICAL
	MEZZANINE MANUFACTURER
11 T. 11 T.	MANUFACTURER
1FR 1TL 1IN.	MINIMUM
	MISCELLANEOUS
	NOT IN CONTRACT NEW
10.	NUMBER
	NOMINAL
I.S.	NEAR SIDE
ITS	NOT TO SCALE
o.c.).D.	ON CENTER OUTSIDE DIAMETER
	OUTSIDE FACE
PNG	OPENING
PP.	OPPOSITE
	ORIENTED STRAND BOARD OVERSIZED HOLE
	POST
Р.А.	POWDER ACTUATED
	PRECAST
PCF	POUNDS PER CUBIC FOOT PENETRATION
	PLATE
	PROPERTY LINE
	PLYWOOD
NL	PANEL POUNDS PER SQUARE FOOT
°SI	POUNDS PER SQUARE FOOT
129	PARALLAM
РТ	POINT PRESSURE TREATED ROOF DRAIN ROUGH OPENING REINFORCING
?.Т.	PRESSURE TREATED
α.D. 2 Ο	ROUGH OPENING
EINF.	REINFORCING
(EQ D	REQUIRED
	REVISION
CHED	RETURN SCHEDULE
SECT.	SECTION
SHT.	SHEET SHEATHING SIMILAR SHRINKAGE CONTROL JOINT
SHTHG	SHEATHING
S.I	SIMILAR Shrinkage control joint
5.0.G.	SHRINKAGE CONTROL JOINT SLAB ON GRADE SPACE
SP.	SPACE
SPEC.	SPECIFICATION
SQ. SSH	SQUARE SHORT SLOTTED HOLE STANDARD
STD	STANDARD
STIFF. STL	STIFFENER
STL	STEEL
TRUCI.	STRUCTURAL SYMMETRICAL
&B	TOP & BOTTOM
&G	TONGUE & GROOVE
ΉK	TONGUE & GROOVE THICKNESS TRUS JOIST
JI	TOP OF CONCRETE
.0.C.	TOP OF FOOTING
.0.J.	TOP OF JOIST TOP OF WALL
.O.W.	TOP OF WALL
J.N.O.	UNLESS NOTED OTHERWISE
′ERT. //	VERTICAL WITH
ÝF	WIDE FLANGE
1/0	WITHOUT
VD V H S	WOOD WELD HEAD STUD
V.H.S. VP	WORK POINT
VΤ	STRUCTURAL TEE
V.W.F.	WELDED WIRE FABRIC

	VEDIER (TRON (ND DATASA	INSPECTION	FREQUENCY	REFERENCED	
Î	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	STANDARD	
	STEEL CONSTRUCTION OTHER T Inspection of welding:	THAN STRUCTUR	AL STEEL		
	a. Cold-formed steel deck: 1) Floor and roof deck welds.		X	AWS D1.3	
	b. Reinforcing steel:		A	Aws D1.5	
	1) Verification of weldability of reinforcing steel other than ASTM A 706.		Х		
	2) Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary	х		AWS D1.4 ACI 318:	
	elements of special structural walls of concrete and shear reinforcement.			Section 3.5.2	
	3) Shear reinforcement. 4) Other reinforcing steel	X	 X		
	STRUCTURAL	STEEL			
Sp	ecial inspections and non-distructive testing of structural steel elem accordance with the quality assurance ins	ments in buildings		ortions thereof shall be in	
	REQUIRED SPECIAL INSPECTIONS OF OPEN-V TYPE	CONTINUOUS	PERIODIC	REFERENCED	
ι.	Installation of open web steel joist and joist girders	INSPECTIONS	INSPECTIONS	STANDARD	
	a. End connections - welding or bolted.		Х	SJI specifications listed in Section 2207.1.	
	b. Bridging - horizontal or diagonal		x	SЛ specifications listed	
	 Standard bridging. Bridging that differs from the SJI specifications listed in 		-	in Section 2207.1.	
	Section 2207.1.		Х		
			CONCRETE CON		
	REQUIRED SPECIAL INSPECTION TYPE	CONTINUOUS	PERIODIC	REFERENCED	IBC REFERENC
Ι.	Inspection of reinforcing steel, including prestressing tendons,	INSPECTIONS	INSPECTIONS	STANDARD ACI 318: 3.5, 7.1-7.7	1908.4
2.	and placement. Reinforcing bar welding:		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		1900.4
	a. Verify weladability of reinforcing bars other than ASTM A706 b. Inspect single-pass fillet welds, maximum 5/16"; and		X X	AWS D1.4 ACI 318:26.6.4	
3.	c. Inspect all other welds. Inspection of anchors cast in concrete.	X	x	ACI 318: 17.8.2	
). 1.	Inspect anchors post-installed in hardened concrete members.				
	a. Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads	х		ACI 318: 17.8.2.4	
5.	 b. Mechanical anchors and adhesive anchors not defineed in 4.a. Verifying use of required design mix. 		x	ACI 318: Ch. 19, 26.4.3,	1904.1, 1904.2,
5.	Prior to concrete placement, fabricate specimens for strength test		A	26.4.4 ASTM C172, ASTM	1908.2, 1908.3
	perform slump and air content test, and determin the temperature of the concrete.	х		C31, ACI 318: 26.4, 26.12	1908.10
7.	Inspection of concrete and shotcrete placement for proper application techniques.	x		ACI 318: 26.5	1908.6, 1908.7, 1908.8
8.	Verify maintenance of specified curing temperature and		X	ACI 318: 26.5.3-	1908.9
).	techniques. Inspect erection of precast concrete members.		X	26.5.5 ACI 318: Ch. 26.8	
10.	Verification in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of		х	ACI 318: 26.11.2	
11.	shores and forms from beams and structural slabs. Inspect formwork for shape, location and dimensions of the	0.2227	x	ACI 318: 26.11.2(b)	
	concrete member being formed.		А	ACI 518. 20.11.2(0)	
	REQUIRED SPECIAL INSPECTIONS AND TE	STS OF SOILS CONTINUOUS	PERIODIC		
	ТҮРЕ	INSPECTIONS	INSPECTIONS		
	Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	((*****))	Х		
	Verify excavations are extended to proper depth and have reached proper material.		Х		
	Perform classification and testing of compacted fill materials. Verify use of proper materials, densities and lift thicknesses	 V	Х		
1	during placement and compaction of compacted fill. Prior to placement of compacted fill, inspect subgrade and verify	X			
	that site has been prepared properly.		X		
-	WOOD		The second second]	
	Wind Exposure Category B, C, or D and Seismic Design Categor Field Gluing of elements of main windforce-resisting system or	ry C, D, E, or F: X	(IBC 1705.11.1, 1	705.12.2)	
2.	seismic force-resisting system. Nailing, bolting, anchoring and other fastening of components		1245- 12 13		
	within the main windforce or seismicforce resisting system, including wood shear walls, diaphragms, drag struts, braces, and	()	X ^(a)		
a.	hold-downs. Special inspection is not required for wood shear walls, shear pan	els and diaphragm	s, including		
	nailing, bolting, anchoring and other fastening to other component seismicforce resisting system, where the fastener spacing of the s	nts of the main win	dforce or		
	on center.	neathing is more in	Ian 4 m.		
	REQUIRED SPECIAL INSPECTIONS OF PLUMBI		L		
	AND ELECTRICAL COMPONEN • Seismic Design Category C, D, E, or F: (IBC 1705.12.6)	18			
	Anchorage of electrical equipment for emergency and standby power systems.		х		
2.	Installation and anchorage of piping systems designed to carry hazardous materials and their associated mechanical units.		Х		
	Installation and anchorage of ductwork designed to carry hazardous materials.		х		
_	Installation and anchorage of vibration isolation systems where the construction documents require a nominal clearance of 1/4		x		
F	inch or less between the equipment support frame and restraint.	ುತ್ರವರ್ಷ	Α		
	Seismic Design Category E or F: Anchorage of other electrical equipment.		Х		
	REQUIRED SPECIAL INSPECTIONS OF ARCHITECT				
_	TYPE	CONTINUOUS INSPECTIONS	PERIODIC INSPECTIONS		
	Wind Exposure Category B, C, or D and Seismic Design Categor Erection and fastening of roof and wall cladding.				
For	Seismic Design Category D, E, or F: Interior and exterior nonbearing walls.		X		
_	Interior and exterior veneer.		X		
	ecial Inspection/Testing Program Notes:			0,1	
	The special inspector shall be a qualified person who shall demon of inspections listed.				
	If necessary, the contractor shall arrange a pre-construction meeti special inspection program.	ing with the Archit	ect, Engineer, Build	ding Official, Testing Age	ency to review the
3.	Duties of the Special Inspector include, but are not limited to: a. Observe the work for conformance with the approved permit p	lans and specificat	ions. Discrepenci	es shall be brought to the	attention of the
	Contractor for correction, if uncorrected, then to the attention	of the Architect, E	ngineer, and Buildi	ing Official.	
	 Issue Inspection Reports for each inspection to the Contractor 	r, Architect Fnom	cel, and Bundling	savenue and a community 1110	Statul UV
	 b. Issue Inspection Reports for each inspection to the Contractor issued in a timely manner. c. The Special Inspector shall submit a final report stating whether 				
		er the work requiri	ng special inspection	on was inspected, whethe	er the work was

- construction documents.



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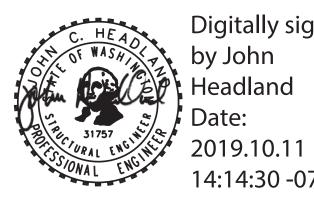
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REVISIONS

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		PERMIT SET	
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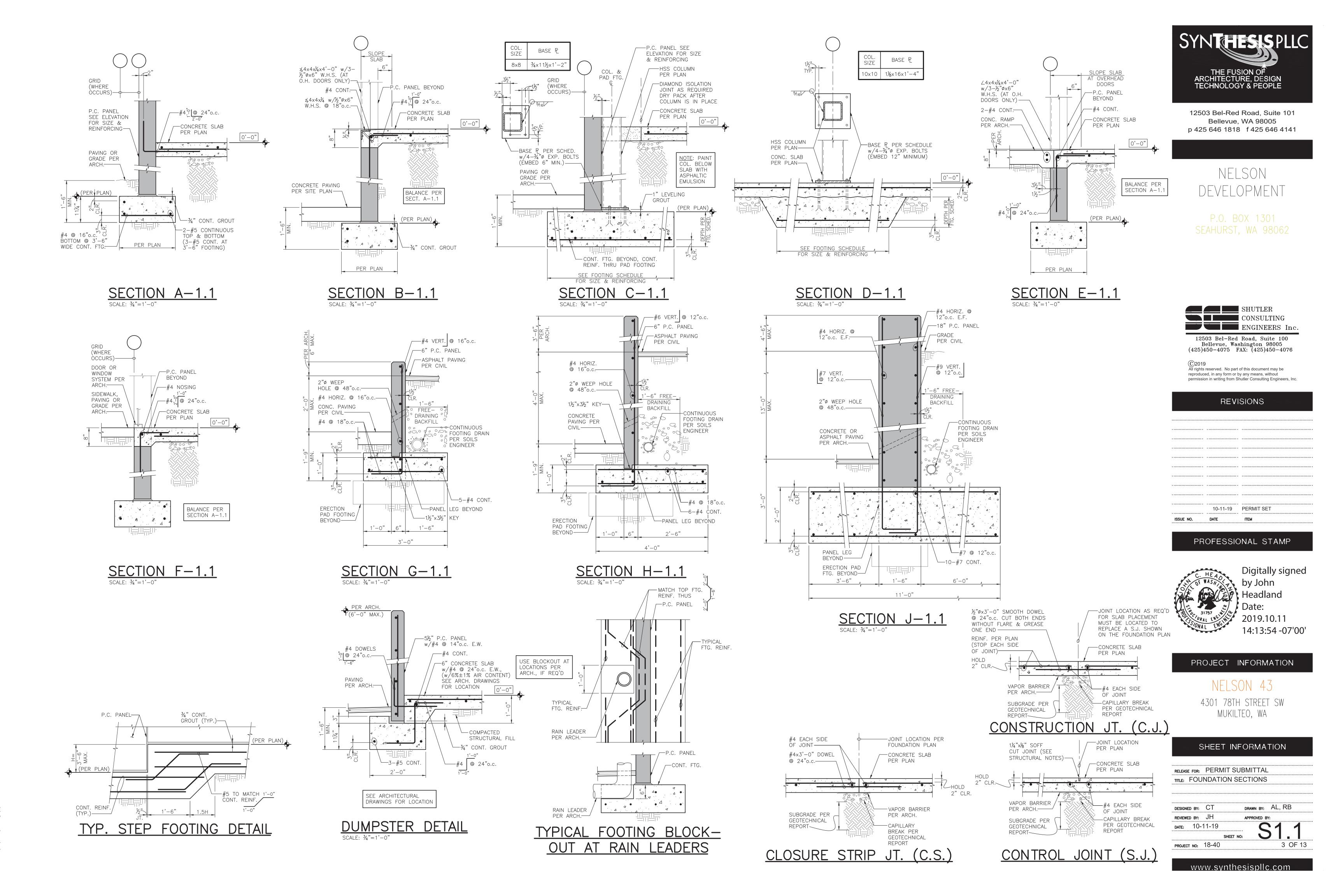
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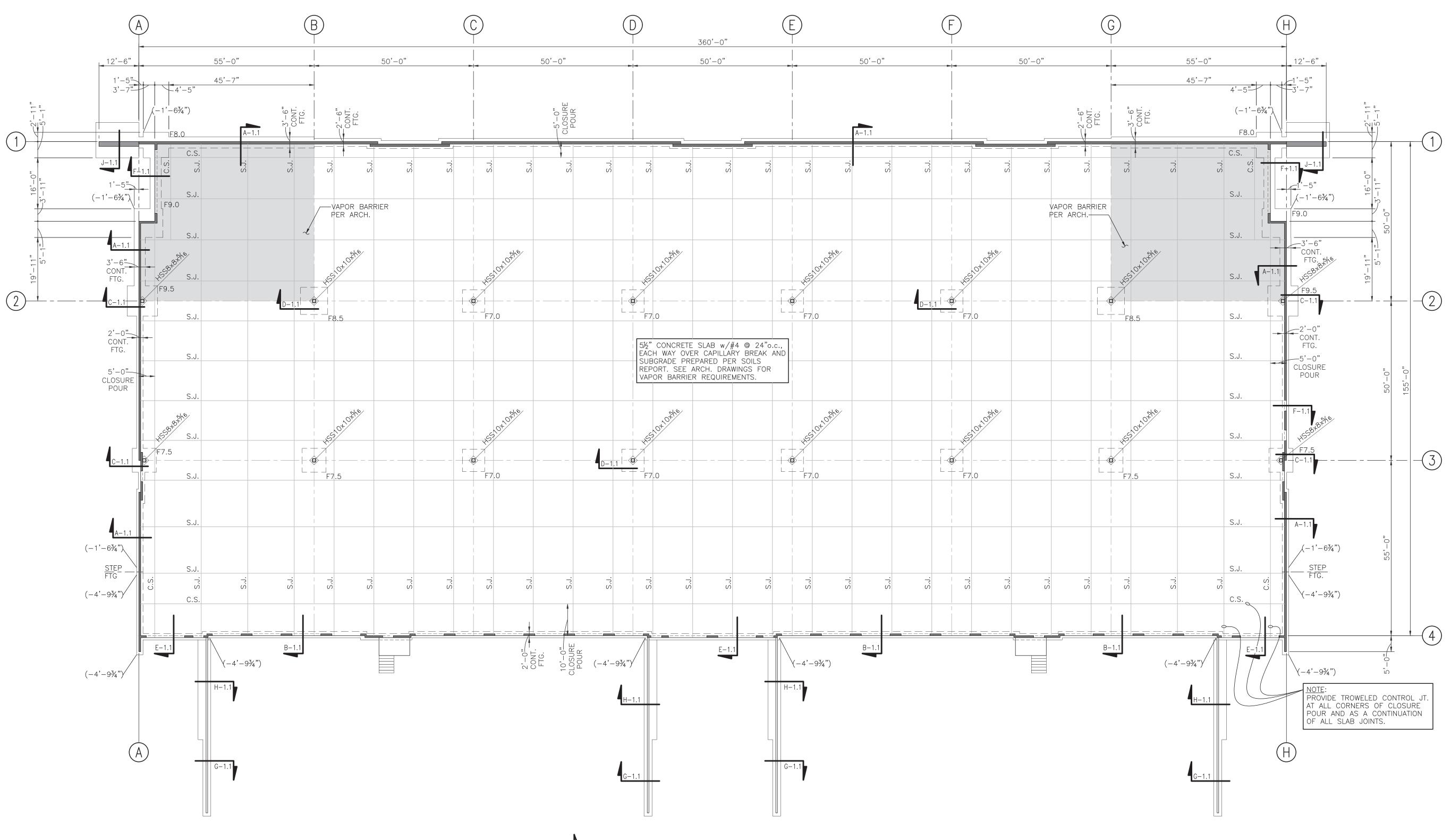
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SHEET INFORMATION

release for: PERI	MIT SUBMITTAL
	NSPECTIONS &
ABBREVIA	ATIONS
designed by: CT	drawn by: AL, RB
reviewed by: JH	APPROVED BY:
date: 10-11-19	
	SHEET NO:

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FOUNDATION PLAN

SCALE: ⅓6"=1'−0"

- NOTES: 1) TOP OF FINISHED FLOOR SLAB AT ELEVATION 561.00' IS REFERENCE DATUM 0'-0''.
 - ELEVATION SHOWN THUS: (-___'-___') INDICATE TOP OF FOOTING ELEVATION BELOW REFERENCE DATUM.
 - BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE 1'-6" MINIMUM BELOW LOWEST ADJACENT GRADE.
 - 4) SEE SHEET S-1.1 FOR STEP FOOTING & SLAB JOINT DETAILS.
 - 5) SEE SHEET S-3.0 FOR FOOTING SCHEDULE.
 - 6) SEE PANEL ELEVATIONS FOR PANEL THICKNESS.



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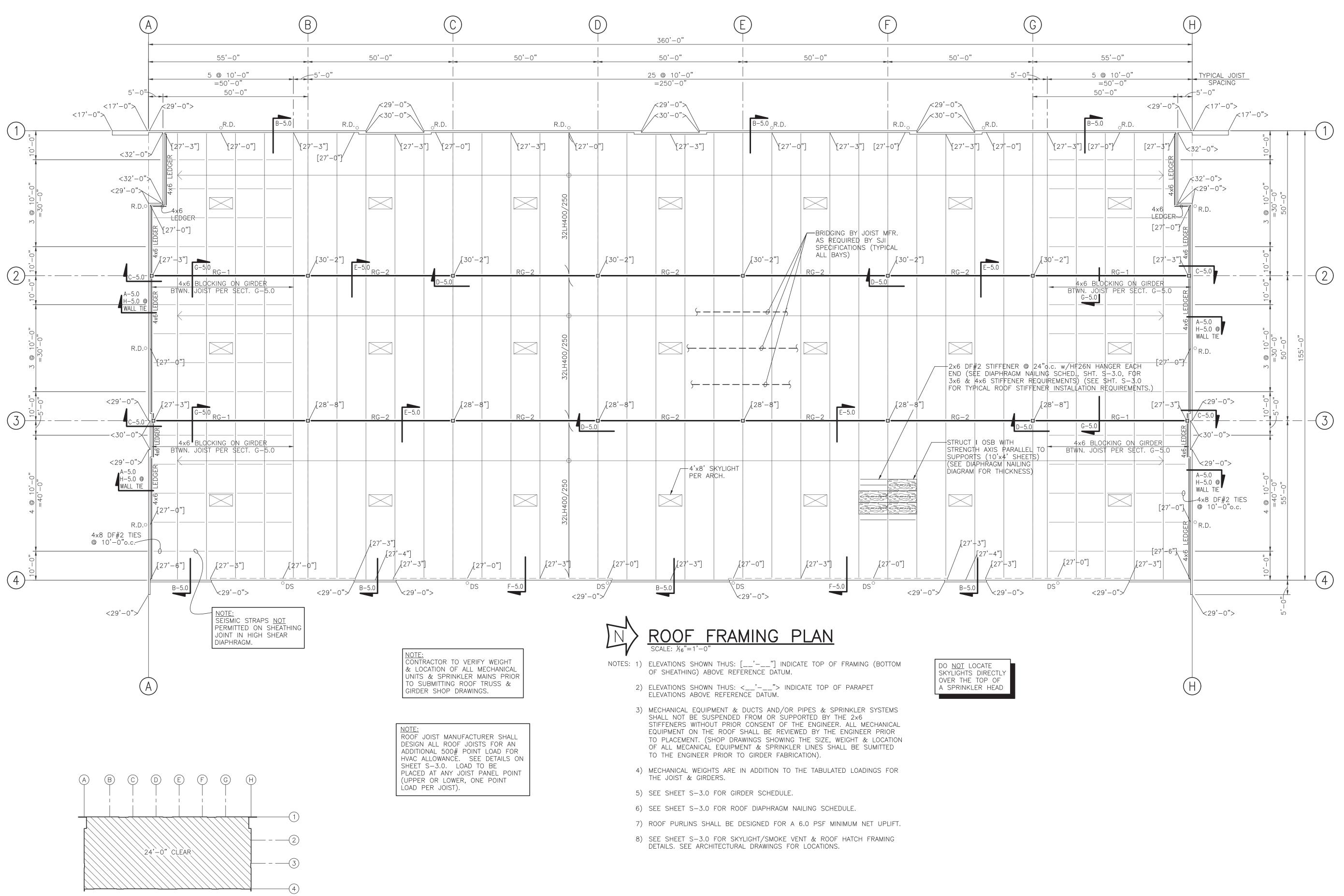
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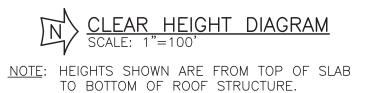
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SHEET INFORMATION

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TITLE: FOU	NDATI	ION PLAN	
DESIGNED BY:	CT	DRAWN BY:	AL, RB
REVIEWED BY:	JH	APPROVED B	Y:
date: 10-	11-19	C	0 0
		SHEET NO:	$\mathbf{V}_{\mathbf{U}}$
PROJECT NO:	18-40		4 OF 13
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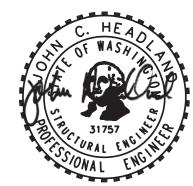
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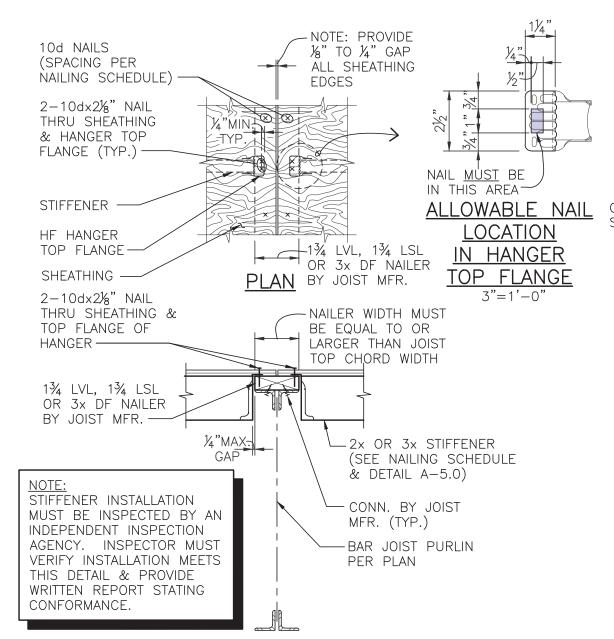
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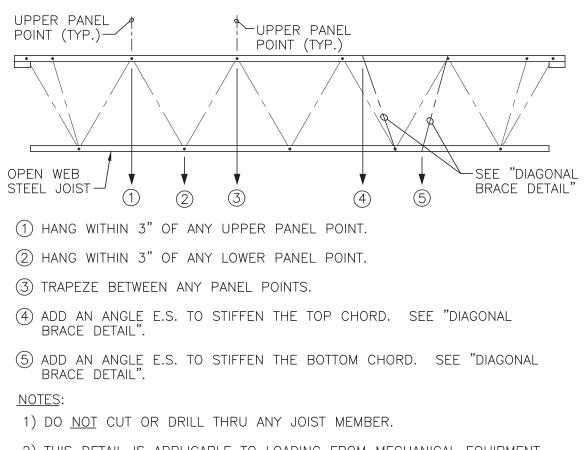
NELSON 43 4301 78TH STREET SW MUKILTEO, WA

SHEET INFORMATION

RELEASE FOR: PER	MIT SUBMITTAL
TITLE: ROOF FR	AMING PLAN
DESIGNED BY: CT	drawn by: AL, RB
REVIEWED BY: JH	APPROVED BY:
date: 10-11-19	C0 1
	SHEET NO: JZ
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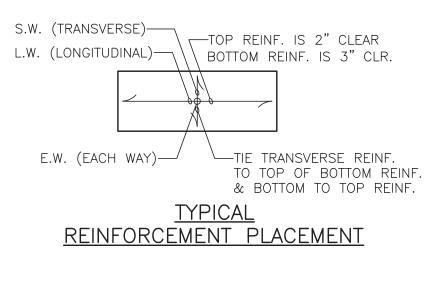
2) THIS DETAIL IS APPLICABLE TO LOADING FROM MECHANICAL EQUIPMENT, SPRINKLER PIPES, ETC. 3) ROOF JOISTS ARE DESIGNED TO SUPPORT AN EQUIVALENT UNIFORM

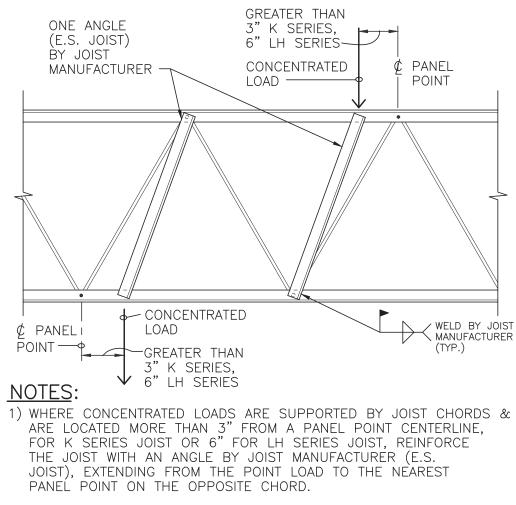
COLLATERAL DEAD LOAD OF 6 POUNDS PER SQUARE FOOT (OF ROOF AREA) FOR THE SUPPORT OF SUSPENDED CEILINGS, DUCTS, SPRINKLER LINEŚ, ETC. (15 PSF TOTAL DEAD LOAD).

TYP. ROOF STIFFENER HANGER LOCATIONS FOR HANGING LOADS INSTALLATION REQUIREMENTS

FOOTING SCHEDULE							
MARK	SIZE	REINFORCEMENT					
F3.0	3'-0"x3'-0"x0'-10"	3-#5 E.W., BOTTOM					
F3.5	3'-6"x3'-6"x0'-10"	3-#5 E.W., BOTTOM					
F4.0	4'-0"x4'-0"x0'-11"	4-#5 E.W., BOTTOM					
F4.5	4'-6"x4'-6"x1'-0"	4-#6 E.W., BOTTOM					
F5.0	5'-0"x5'-0"x1'-1"	4-#6 E.W., BOTTOM					
F5.5	5'-6"x5'-6"x1'-2"	5-#6 E.W., BOTTOM					
F6.0	6'-0"x6'-0"x1'-4"	6-#6 E.W., BOTTOM					
F6.5	6'-6"x6'-6"x1'-5"	7-#6 E.W., BOTTOM					
F7.0	7'-0"x7'-0"x1'-6"	7-#6 E.W., BOTTOM					
F7.5	7'-6"x7'-6"x1'-7"	8-#6 E.W., BOTTOM					
F8.0	8'-0"x8'-0"x1'-8"	9-#6 E.W., BOTTOM					
F8.5	8'-6"x8'-6"x1'-9"	8-#7 E.W., BOTTOM					
F9.0	9'-0"x9'-0"x1'-10"	8-#7 E.W., BOTTOM					
F9.5	9'-6"x9'-6"x1'-11"	9-#7 E.W., BOTTOM					
F10.0	10'-0"x10'-0"x2'-0"	10-#7 E.W., BOTTOM					

NOTE: SOME FOOTINGS IN SCHEDULE MAY NOT BE USED.





2) REMOVE LOAD FROM JOIST PRIOR TO WELDING ANGLE.

DIAGONAL BRACE DETAIL

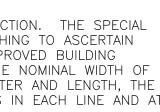
HIGH SHEAR ROOF DIAPHRAGM NAILING SCHEDULE						
MARK	SHEATHING THICKNESS & GRADE	STIFFENER AT PLYWOOD JOINT	NUMBER OF ROWS OF NAILS	NAIL SPACING ALONG CONTINUOUS PANEL EDGES	BOUNDARY NAILING	ALLOWABLE SHEAR
	¹⁵ / ₃₂ " Struct I	4x6	2	2 ROWS @ 4"o.c.	2 ROWS @ 4"o.c.	915# _{FT}
VI	¹⁵ / ₃₂ " Struct I	4x6	2	2 ROWS @ 3"o.c.	2 ROWS @ 3"o.c.	1165# _{FT}
VII	¹⁵ 32" Struct I	4x6	2	2 ROWS © 2½"o.c.	2 ROWS © 2½"o.c.	1290#ft
	. 19 ₃₂ " Struct I	4x6	2	2 ROWS @ 2½"o.c.	2 ROWS @ 2½"o.c.	1440# _{FT}

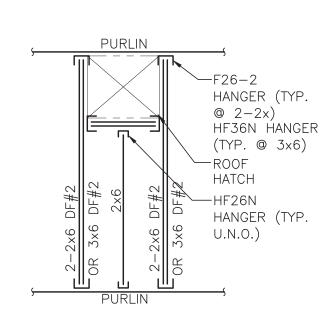
NOTES: 1) ALL NAILS TO BE 10d COMMON (.1480x2" MINIMUM LENGTH).

- 2) SPACE NAILS @ 12"o.c. AT ALL INTERMEDIATE FRAMING MEMBERS.
- 3) PROVIDE 2 ROWS 10d @ 21/2"o.c., STAGGERED AT ALL DIAPHRAGM BOUNDARIES.
- 4) provide 4x6 stiffener w/F46 hanger each end at all panel edges in Mark $\langle V \rangle$ $\langle VI \rangle$, $\langle VII \rangle$ and $\langle VIII \rangle$.
- 5) PROVIDE 4x8 STIFFENER w/B48 HANGER EACH END AT ALL SEISMIC STRAPS (SEE SECTION H-5.0). SEISMIC STRAPS NOT PERMITTED ON SHEATHING JOINT AT EXTERIOR WALL IN HIGH SHEAR DIAPHRAGM.
- 6) AT DRAG STRUT IN HIGH SHEAR NAILING REGION, PROVIDE 2 ROWS 10d COMMON @ 21/2"o.c. STAGGERED EACH SIDE OF JOINT PER HIGH SHEAR DIAPHRAGM NAILING DETAIL
- 7) HIGH-LOAD DIAHRAGMS MUST BE INSTALLED WITH SPECIAL INSPECTION. THE SPECIAL INSPECTOR MUST INSPECT THE STRUCTURAL WOOD PANEL SHEATHING TO ASCERTAIN WHETHER IT IS OF THE GRADE & THICKNESS SHOWN ON THE APROVED BUILDING PLANS. ADDITIONALLY, THE SPECIAL INSPECTOR MUST VERIFY THE NOMINAL WIDTH OF FRAMING MEMBERS AT ADJOINING PANEL EDGES, THE NAIL DIAMETER AND LENGTH, THE NUMBER OF FASTENER LINES, THE SPACING BETWEEN FASTENERS IN EACH LINE AND AT EDGE MARGINS AGREE WITH THE APPROVED BUILDING PLANS.

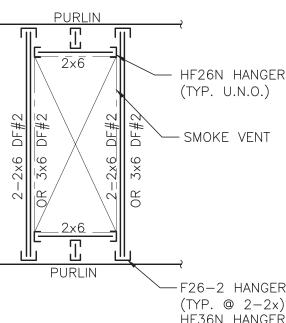






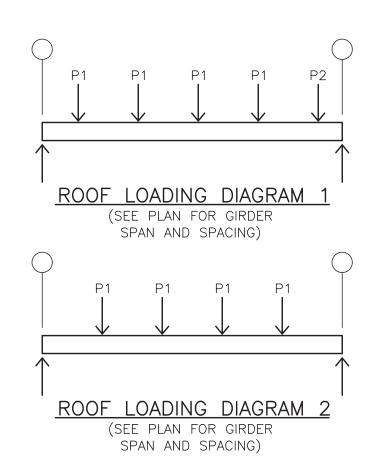


ROOF HATCH FRAMING DETAIL



(TYP. @ 2-2x) HF36N HANGER (TYP. @ 3x6)





ROOF GIRDER DESIGN CRITERIA								
MARK DEPTH NOMENCLATURE DIAGRAM) P2	NET	
		NUMBE		T.L.	L.L.	T.L.	L.L.	UPLIFT
RG-1	32"/50"	32/50G6N SPECIAL	1	21.8 ^K	13.1 ^ĸ	16.4 ^ĸ	9.8 ^ĸ	5 PSF
RG-2	48"	48G5N21.8 ^K	2	21.8 ^K	13.1 ^K			5 PSF
ROOF	GIRDER SCHE	DULE NOTES						

2) MECHANICAL EQUIPMENT LOADS, ROOF TOP UNIT LOADS & SPRINKLER MAIN LOADS ARE IN ADDITION TO LOADS SHOWN.

TOTAL IF REQUIRED.

IN SCHEDULE.

ROOF DIAPHRAGM NAILING SCHEDULE						
MARK	SHEATHING	STIFFENER AT PLYWOOD JOINT	CONTINUOUS EDGES	'OTHER' EDGE	ALLOWABLE SHEAR	
$\langle \overline{I} \rangle$	¹⁵ ⁄ ₃₂ "	2x6	6"o.c.	6"o.c.	320#/FT	
II	¹⁵ ⁄ ₃₂ "	2x6	4"o.c.	4"o.c.	425#/FT	
	¹⁵ ⁄ ₃₂ "	2x6	3"o.c.	3"o.c.	568#/FT	
IV	¹⁵ ⁄ ₃₂ "	3x6	3"o.c.	3"o.c.	640#/FT	

NOTES:

1) ALL NAILS TO BE 10d COMMON (0.148"øx2" MINIMUM LENGTH).

2) SPACE NAILS @ 12"o.c. AT ALL INTERMEDIATE FRAMING MEMBERS.

3) PROVIDE 2 ROWS 10d @ 2½"o.c., STAGGERED AT ALL DIAPHRAGM BOUNDARIES.

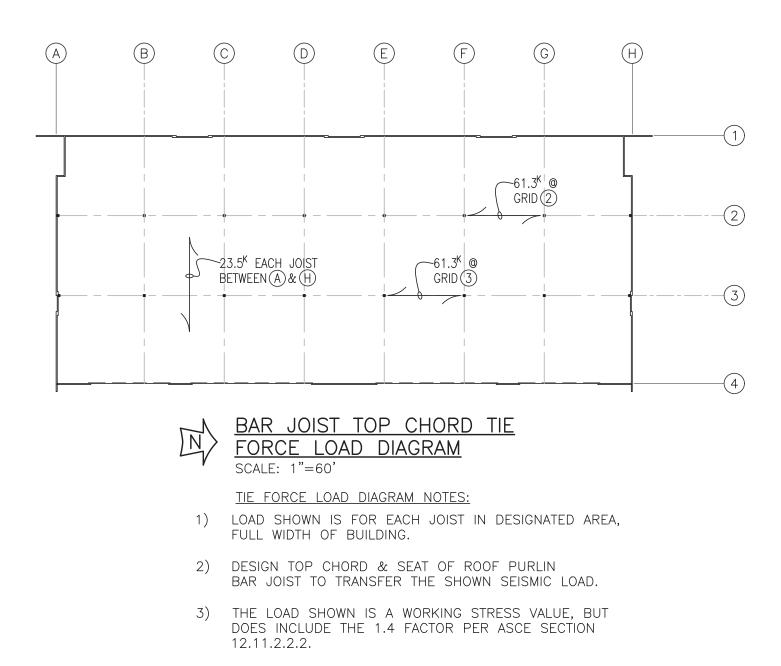
4) PROVIDE 4x8 STIFFENER w/B48 HANGER EACH END AT ALL SEISMIC STRAPS (SEE SECTION H-5.0) (SEE HIGH SHEAR ROOF DIAPHRAGM NOTES FOR ADDITIONAL REQUIREMENTS).

5) PROVIDE 2 ROWS 10d COMMON @ 2½"o.c., STAGGERED EACH SIDE OF JOINT AT DRAG STRUTS. (SEE HIGH SHEAR ROOF DIAPHRAGM NOTES FOR ADDITIONAL DRAG STRUT NAILING REQUIREMENTS.

6) IF NAILS LONGER THAN 2" IN LENGTH ARE USED, THEN 3x6 STIFFENERS (WITH HF36N HANGERS EACH END) ARE REQUIRED AT PANEL EDGES IN MARK(III).

7) PROVIDE 3x6 STIFFENERS WITH HF36N HANGERS EACH END AT ALL PANEL EDGES IN MARK $\langle IV \rangle$

8) PROVIDE 1/8" TO 1/4" GAP AT ALL SHEATHING EDGES.



<u>GIRDER SCHEDULE NUTES</u>

1) LIVE LOAD DEFLECTIONS SHALL BE LIMITED TO SPAN/360. TOTAL LOAD DEFLECTIONS SHALL BE LIMITED TO SPAN/240.

3) DEPTH OF JOIST GIRDER BEARING TO BE 71/2".

4) TOTAL LOADS SHOWN INCLUDE AN 80 PLF ALLOWANCE FOR THE GIRDER WEIGHT. ADJUST THE

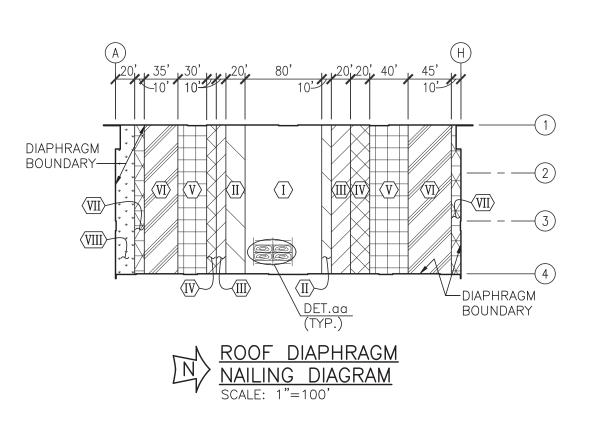
5) MECHANICAL UNIT LOADS SHOWN ON THE DRAWINGS ARE TO BE ADDED TO THE LOADING INDICATED

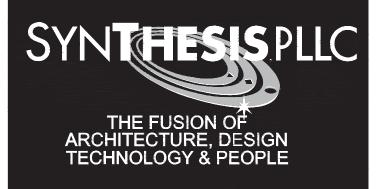
6) ROOF GIRDERS INDICATED BY xx/yy DEPTH SHALL BE TAPERED FROM xx" DEPTH AT THE LOWER ROOF ELEVATION COLUMN TO yy" DEPTH AT THE HIGHER ROOF ELEVATION COLUMN. 7) ALL GIRDERS WITH A STRAIGHT TAPER END-TO-END TO MAINTAIN CLEAR HEIGHTS MAY, AT THE SUBCONTRACTOR'S OPTION, BE DESIGNED & FABRICATED AT A UNIFORM DEPTH EQUAL TO THE

TABULATED SHALLOW END DEPTH. 8) SEE THIS SHEET FOR GIRDER TOP CHORD AXIAL LOADS.

4'-0"x10'-0" SHEETS OF PLYWOOD	CONT. EDGES IN BOTH DIRECTIONS
PLYWD JOINTS NEED NOT BE STAGGERED	INTERMEDIATE MEMBERS
DETAIL	<u>aa</u> spacing per
134 ² - 134 ¹¹	SCHED. (TYP.)
<u>HIGH SHEAR I</u> NAILING DETA	DIAPHRAGM
PANEL EDGE S S S S S S S S S S S S S S S S S S S	SPACING PER SCHED. (TYP.)
BOUNDARY	NAILING:

BOUNDARY NAILING: 2 ROWS STAGGERED





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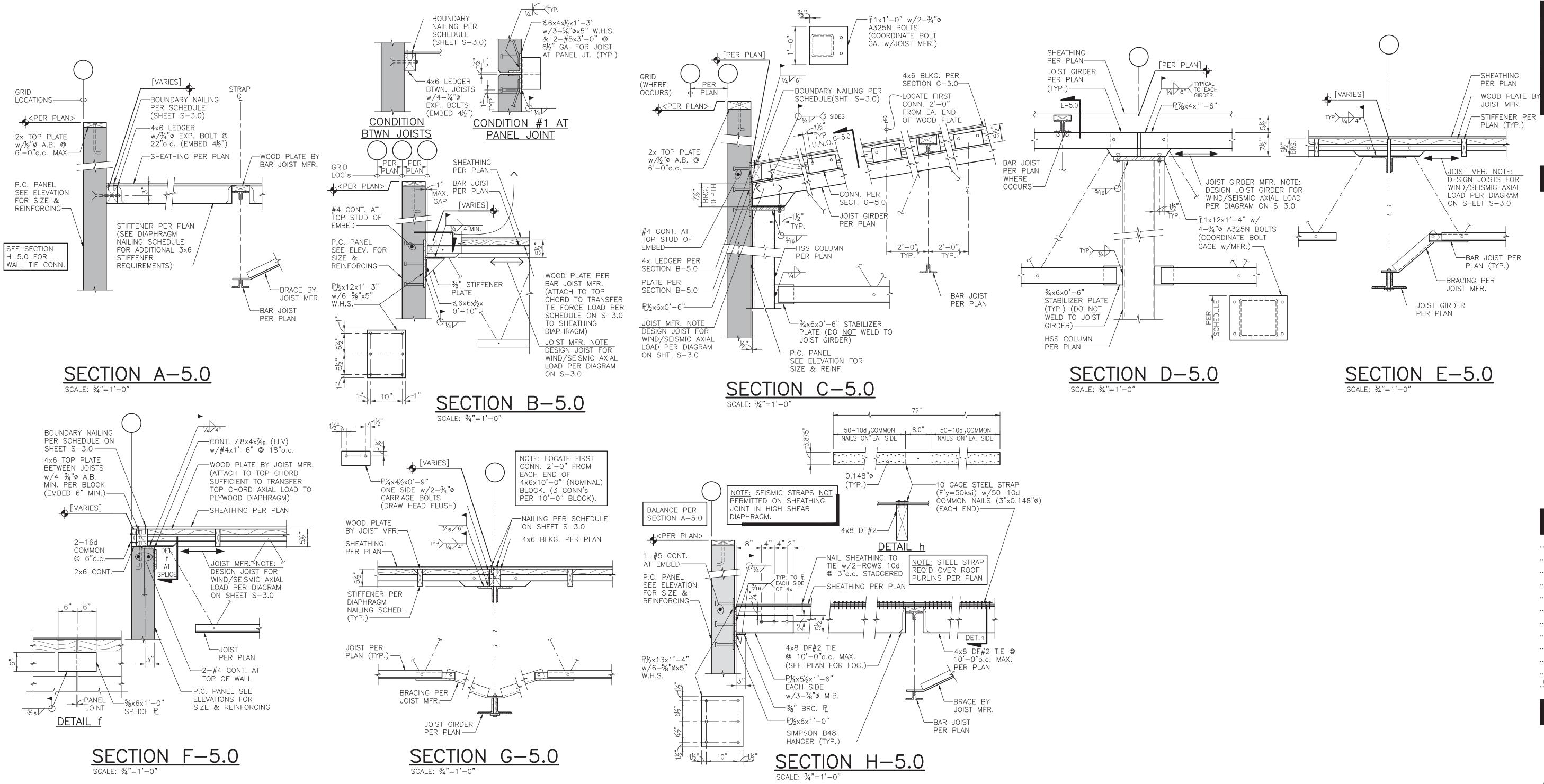


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PROJECT INFORMATION

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TITLE: SCHEDULES AN	ID DIAGRAMS
designed by: CT	drawn by: AL, RB
reviewed by: JH	APPROVED BY:
date: 10-11-19	C_{2}
SHEET N	
project no: 18-40	6 OF 13
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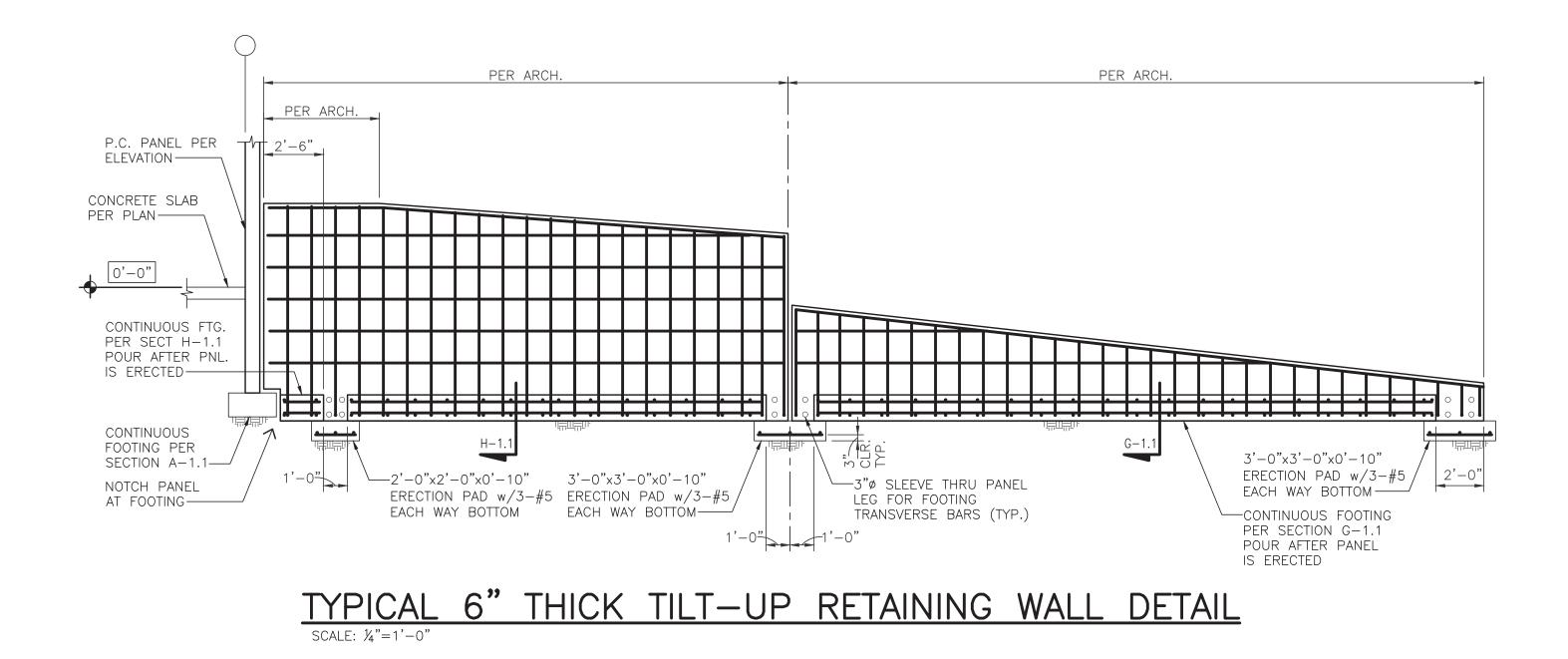
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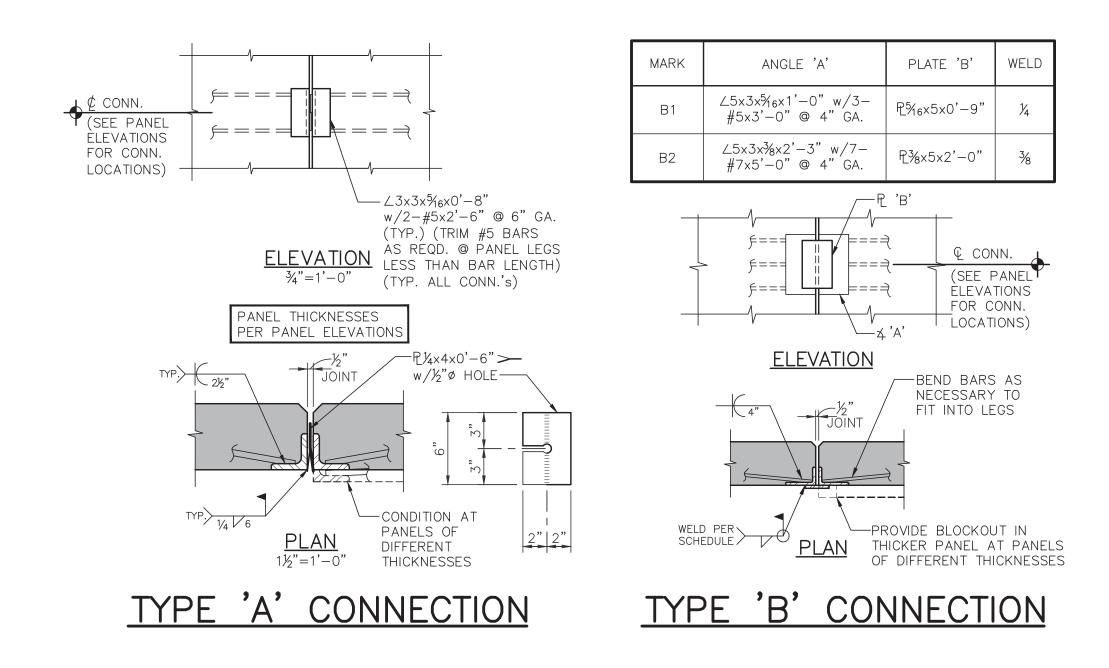
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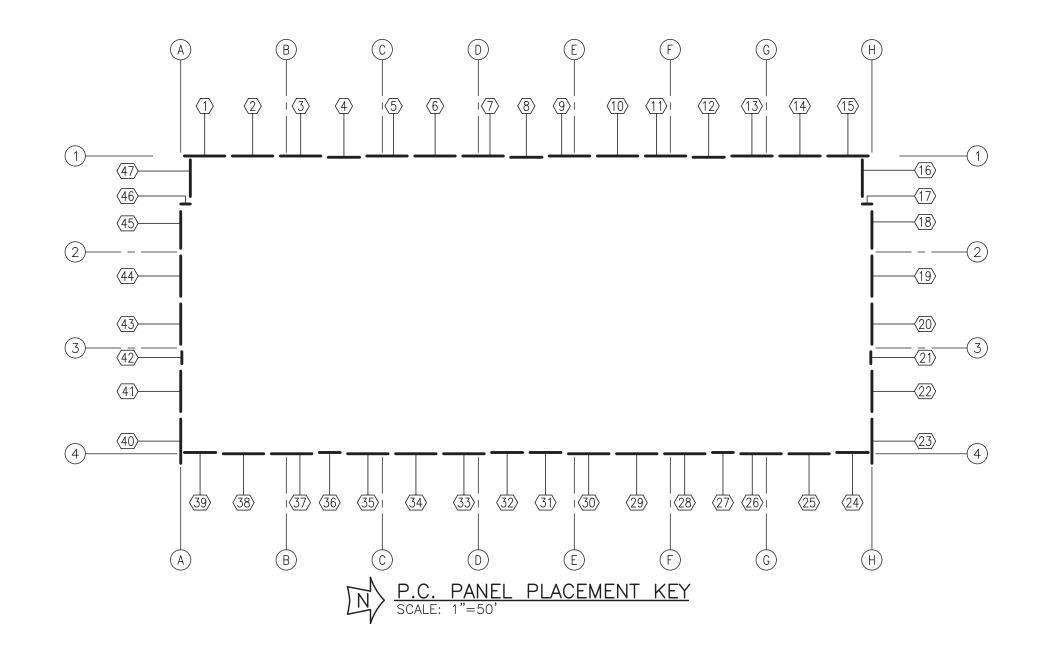
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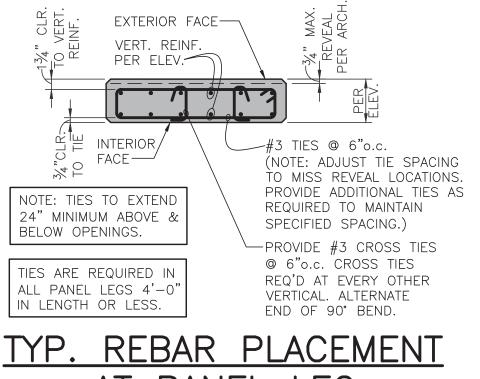
RELEASE FOR: PERMIT SUBMITTAL TITLE: ROOF FRAMING SECTIONS DESIGNED BY: CT DRAWN BY: AL, RB REVIEWED BY: JH APPROVED BY: DATE: 10-11-19 SHEET NO: 5500 PROJECT NO: 18-40 7 OF 13

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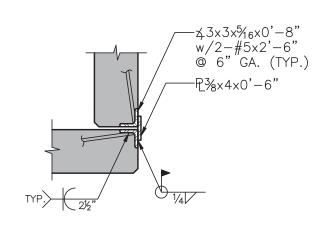




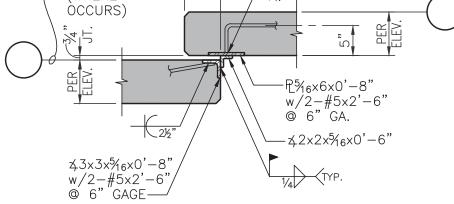


SCHEDULE FOR TYPE 'F' CONNECTION

MARK	ANGLE 'A'	plate 'B'	PLATE 'C'	WELD
F1	≴6x4x½x1'-0 w/3-#5 x3'-0" & 2-¾"øx5" W.H.S.	₽%x4x0'−9"	₽½×6×1'−0" w/3−#5 ×3'−0" & 2−¾"ø×5" W.H.S.	1/4 9" LONG
F2	∡ 6x4x½x2'-3" w/7-#7 x5'-0" & 4-¾"øx5" W.H.S.	₽%×4×2'−0"	₽½×6×2'−3" w/7−#7 ×5'−0" & 4−¾"ø×5" W.H.S.	3% 24" LONG



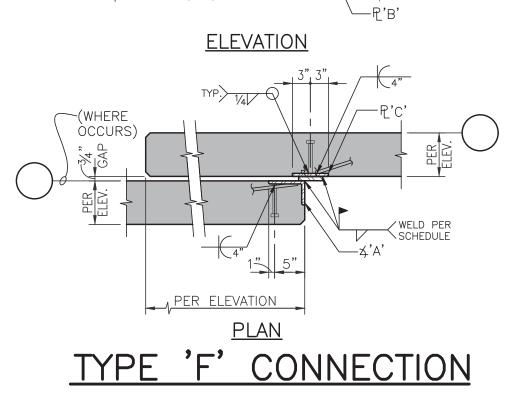
TYPE 'D' CONNECTION ELEV. ~(WHERE

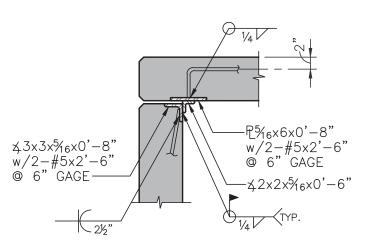






, 5×4×½×1′−0 w/3−#5 x3′−0" & 2−¾"øx5" W.H.S.	₽%×4×0'−9"	₩½×6×1'−0″ w/3−#5 ×3'−0″ & 2−¾″ø×5″ W.H.S.	9
∡ 6x4x½x2'-3" w/7-#7 x5'-0" & 4-¾"øx5" W.H.S.	₽‰x4x2'−0"	₽½×6×2'−3" w/7−#7 ×5'−0" & 4−¾"ø×5" W.H.S.	24
2" 4"GA.			





TYPE 'C' CONNECTION



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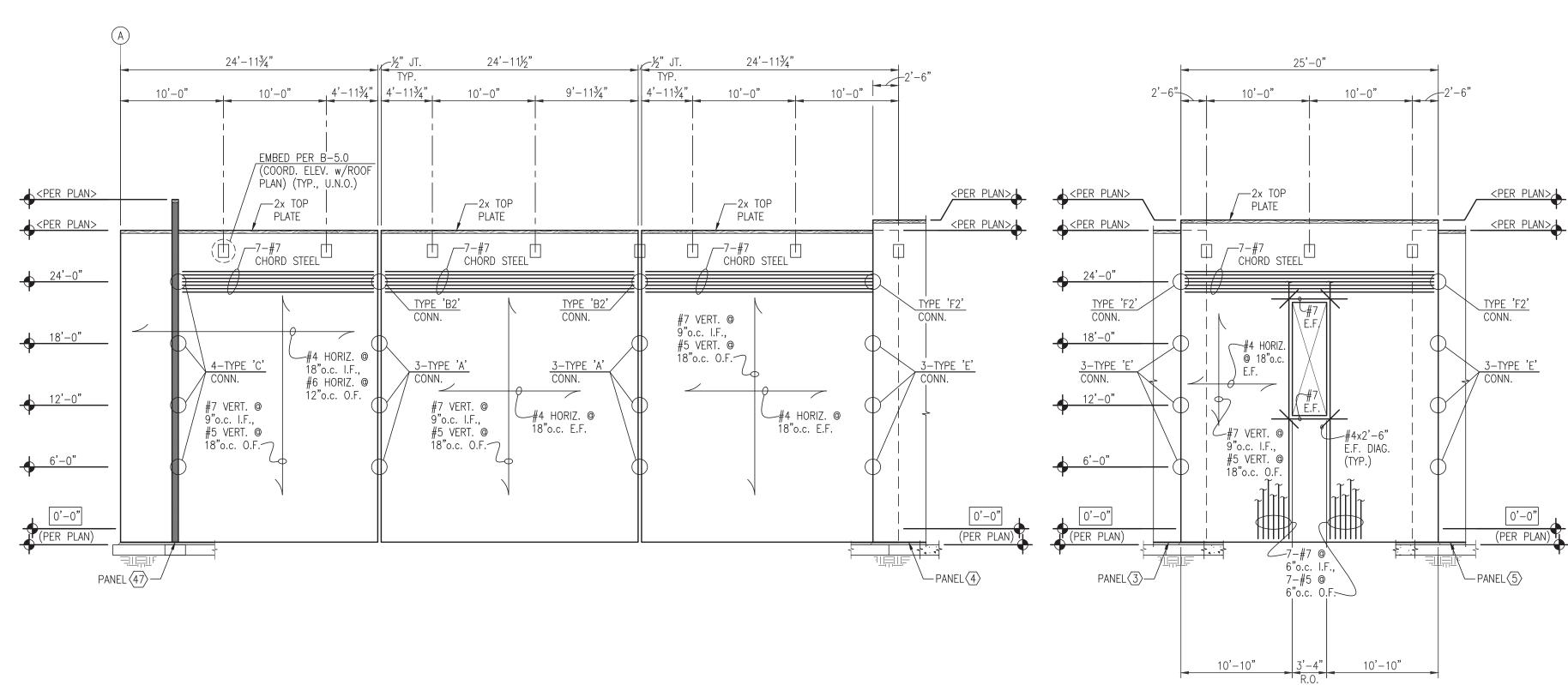
RELEASE FOR: PERMIT SUBMITTAL **TITLE:** PANEL CONNECTIONS, PANEL DETAILS & PANEL KEY designed by: CT DRAWN BY: AL, RB

reviewed by: JH APPROVED BY: S6.0 date: 10-11-19 SHEET NO: project no: 18-40 8 OF 13

PANEL ELEVATIONS (EXTERIOR FACE) -THICKNESS PER PANEL ELEVATIONS -VERT. REINF. PER PANEL ELEVATIONS (TYPICAL) INTERIOR FACE TYPICAL SINGLE MAT REBAR REPLACEMENT HORIZ. REINF. PER PANEL ELEVATIONS (TIED TO INSIDE -EXTERIOR FACE OF VERT. REINF.)-CLR VER ¾″ MAX. REVEAL PER ARCH. THICKNESS -----PER PANEL ELEVATIONS ЧСГ <u>NOTE</u>: Where vert. bars are - 0 <u>-</u> ON EACH FACE (E.F.) & TYP. MAT IS CENTERED, PROVIDE RISER BARS @ -VERT. REINF. PER 48"o.c. MAX. TO MAIN-PANEL ELEVATIONS TAIN CLEAR DIMENSIONS (TYPICAL) TYPICAL DOUBLE MAT **REBAR REPLACEMENT** PRECAST PANEL REINFORCING NOTES: 1) THICKNESS INDICATED IS MINIMUM THICKNESS ALLOWED w/3/4" DEEP REVEAL IN PANELS INDICATED ONLY. DEEPER REVEALS, EXPOSED AGGREGATE ANDR RAKED ARCHITECTURAL FINISHES ARE NOT ALLOWED WITHOUT PRIOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER. 2) PROVIDE 1-#7 OR 2-#5 MINIMUM HORIZONTAL AT TOP & BOTTOM OF ALL OPENINGS (EXTEND 2'-0" BEYOND OPENINGS) UNLESS NOTED OTHERWISE. 3) PROVIDE #4x2'-6" DIAGONALS AT CORNERS OF ALL OPENINGS. #4x2'-6" DIAGONALS EACH FACE AT CORNERS OF ALL OPENINGS IN PANELS OVER 74" THICK. 4) PANEL ELEVATIONS ARE VIEWED FROM INSIDE LOOKING OUT, UNLESS NOTED OTHERWISE. 5) SEE PANEL ELEVATIONS FOR PANEL THICKNESS. 6) CONTRACTOR IS TO VERIFY ALL DIMENSIONS IN FIELD PRIOR TO FORMING PANELS.

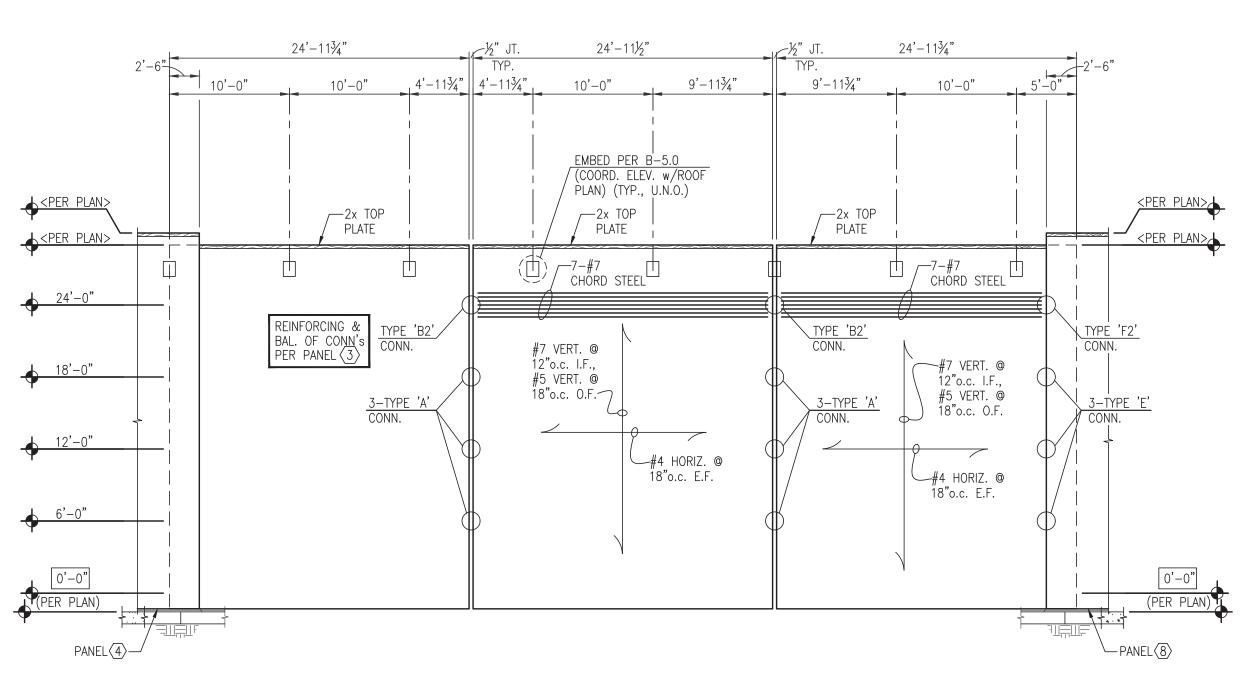
-HORIZ. REINF. PER

- 7) SHOP DRILL $\frac{1}{4}$ " DIAMETER AIR-RELEASE HOLES IN EMBEDMENTS AT 6" CENTERS TO DISPLACE ALL VOIDS.
- 8) REINFORCING IS TO BE PLACED PER "TYPICAL SINGLE MAT REBAR PLACEMENT" DETAIL UNLESS INDICATED AS EACH FACE ("E.F."). SEE "TYPICAL DOUBLE MAT REBAR PLACEMENT" DETAIL FOR REBAR PLACEMENT INDICATED AS EACH FACE ("E.F.")

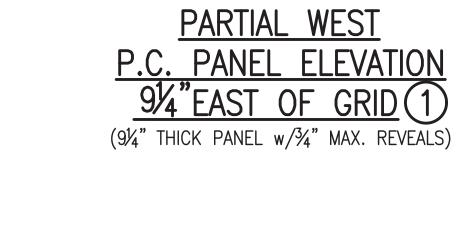




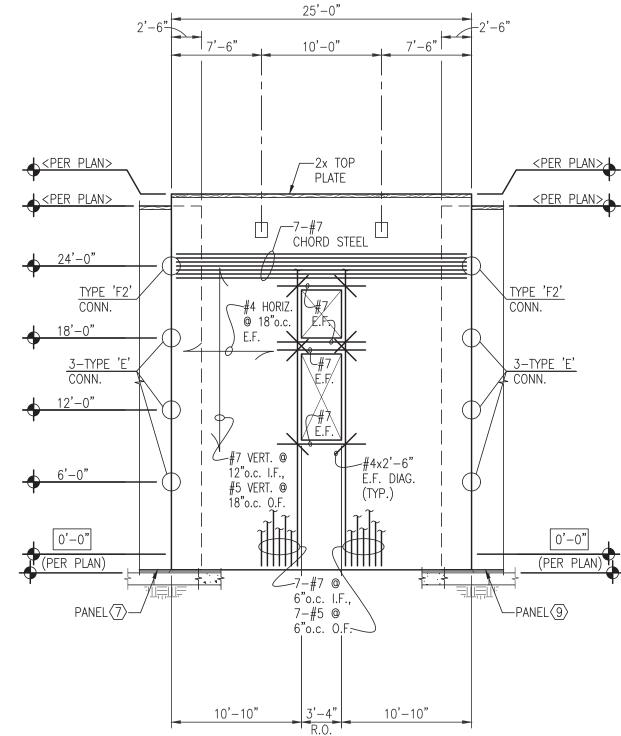








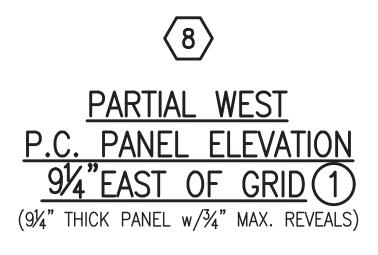
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PARTIAL WEST P.C. PANEL ELEVATIONS AT GRID (1) (91/4" THICK PANELS w/3/4" MAX. REVEALS)











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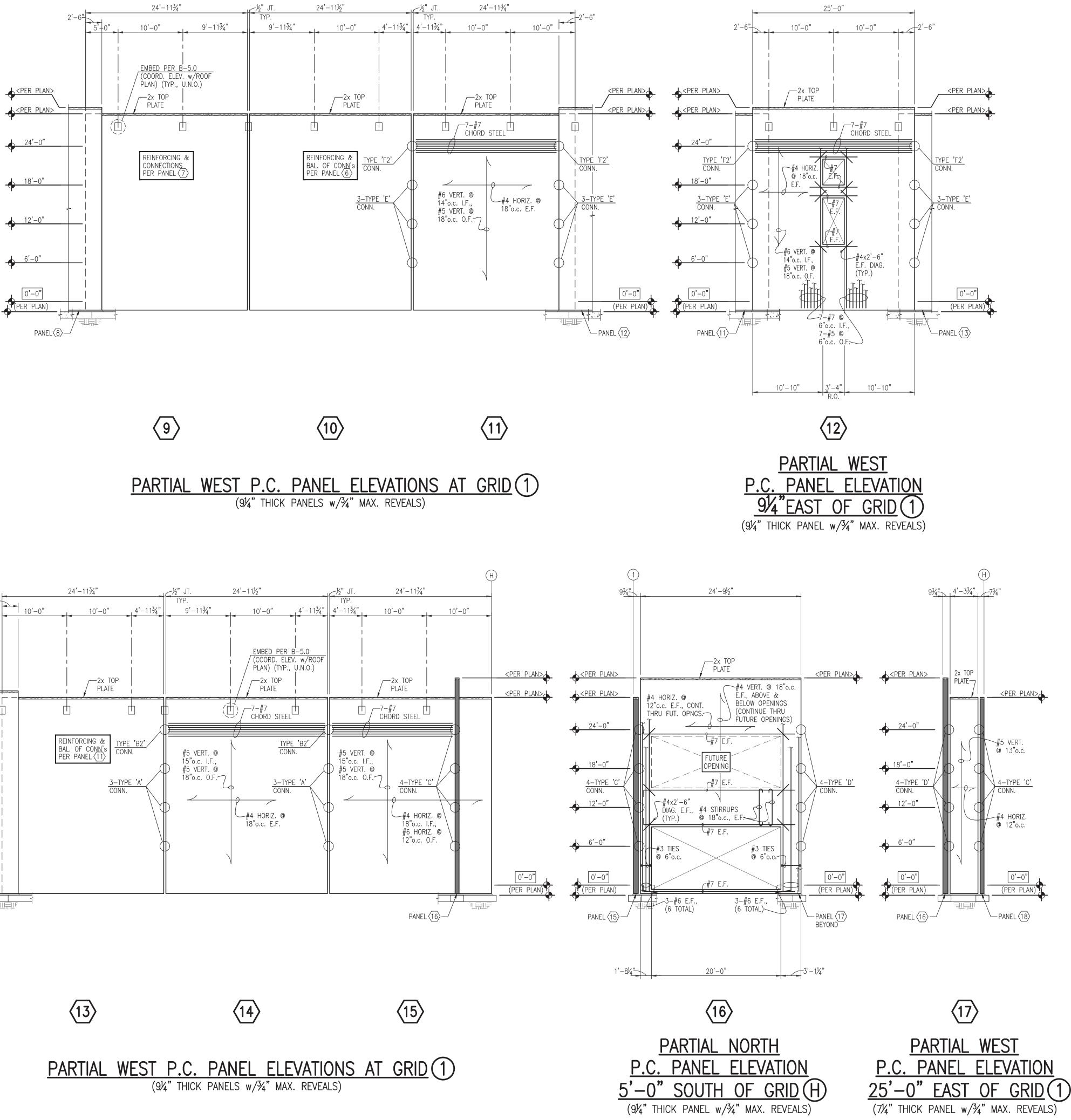
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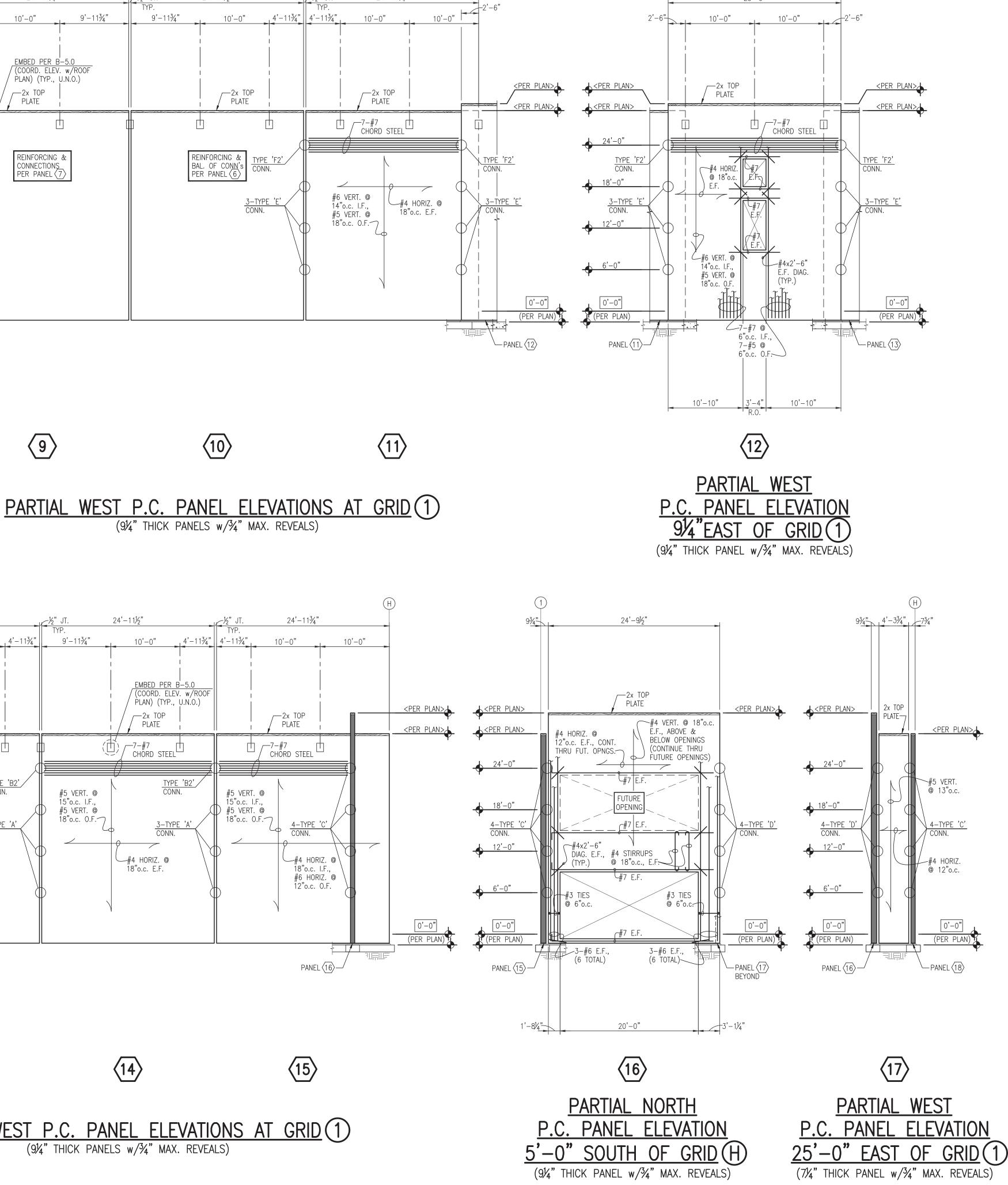
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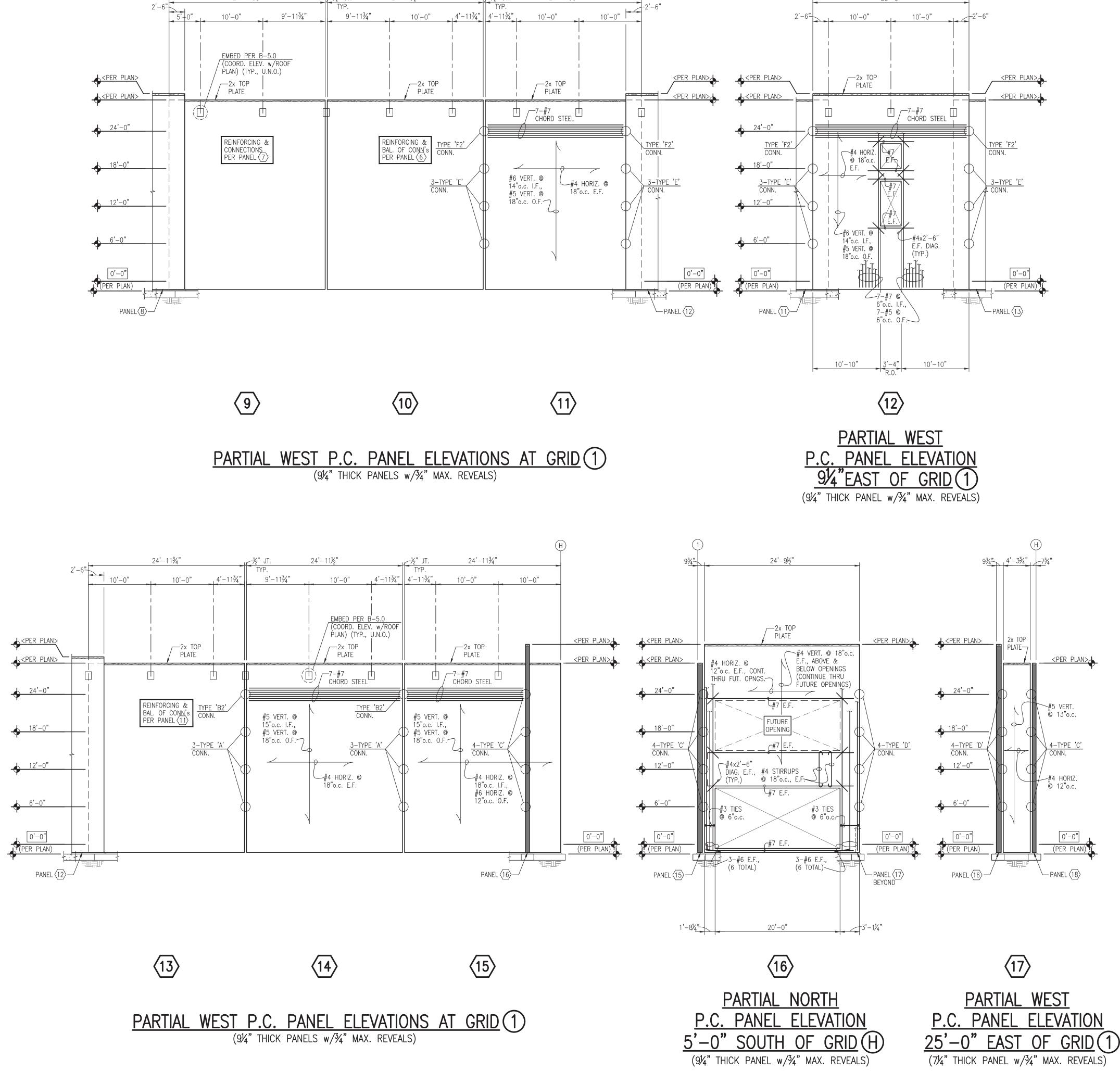
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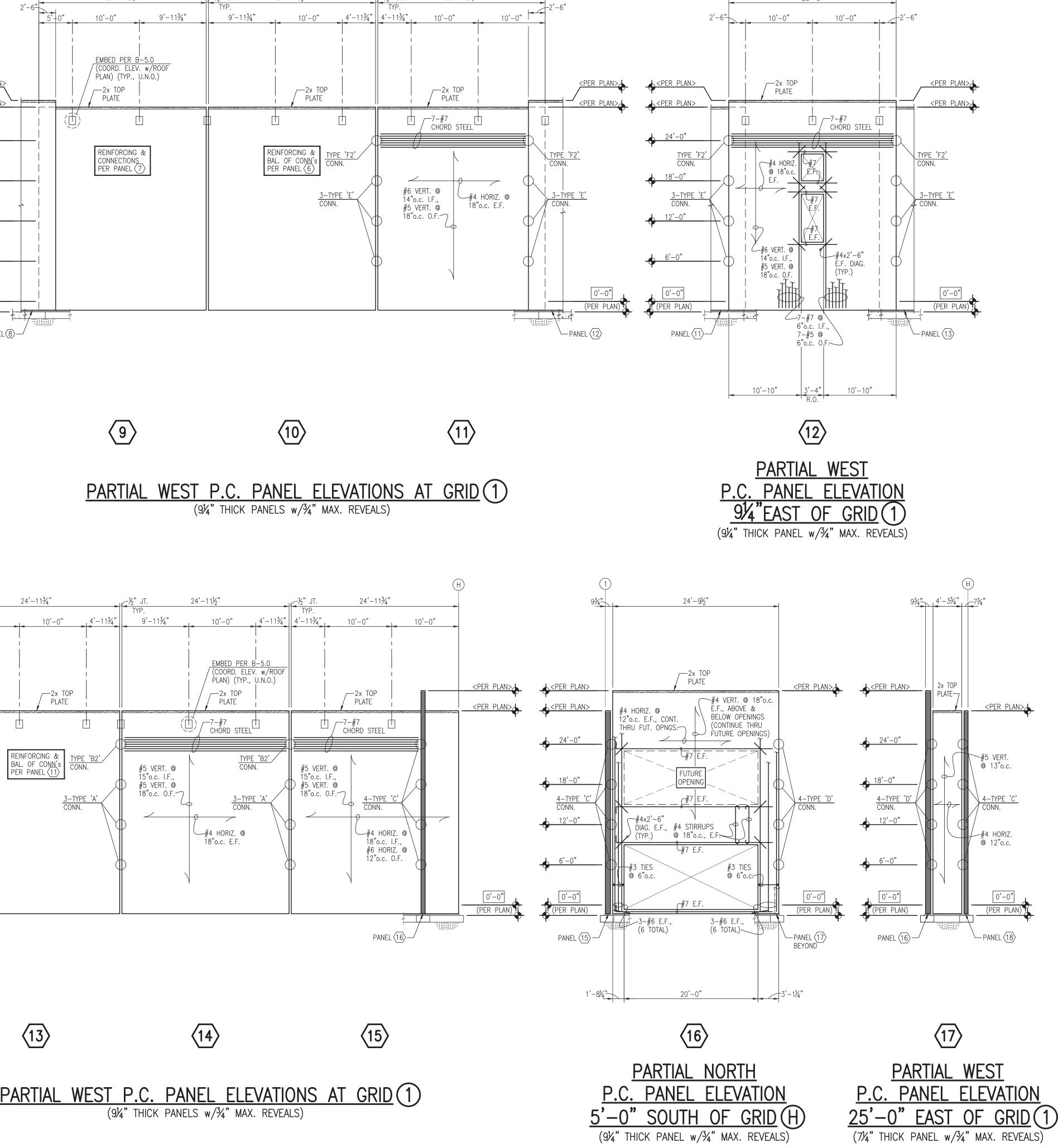
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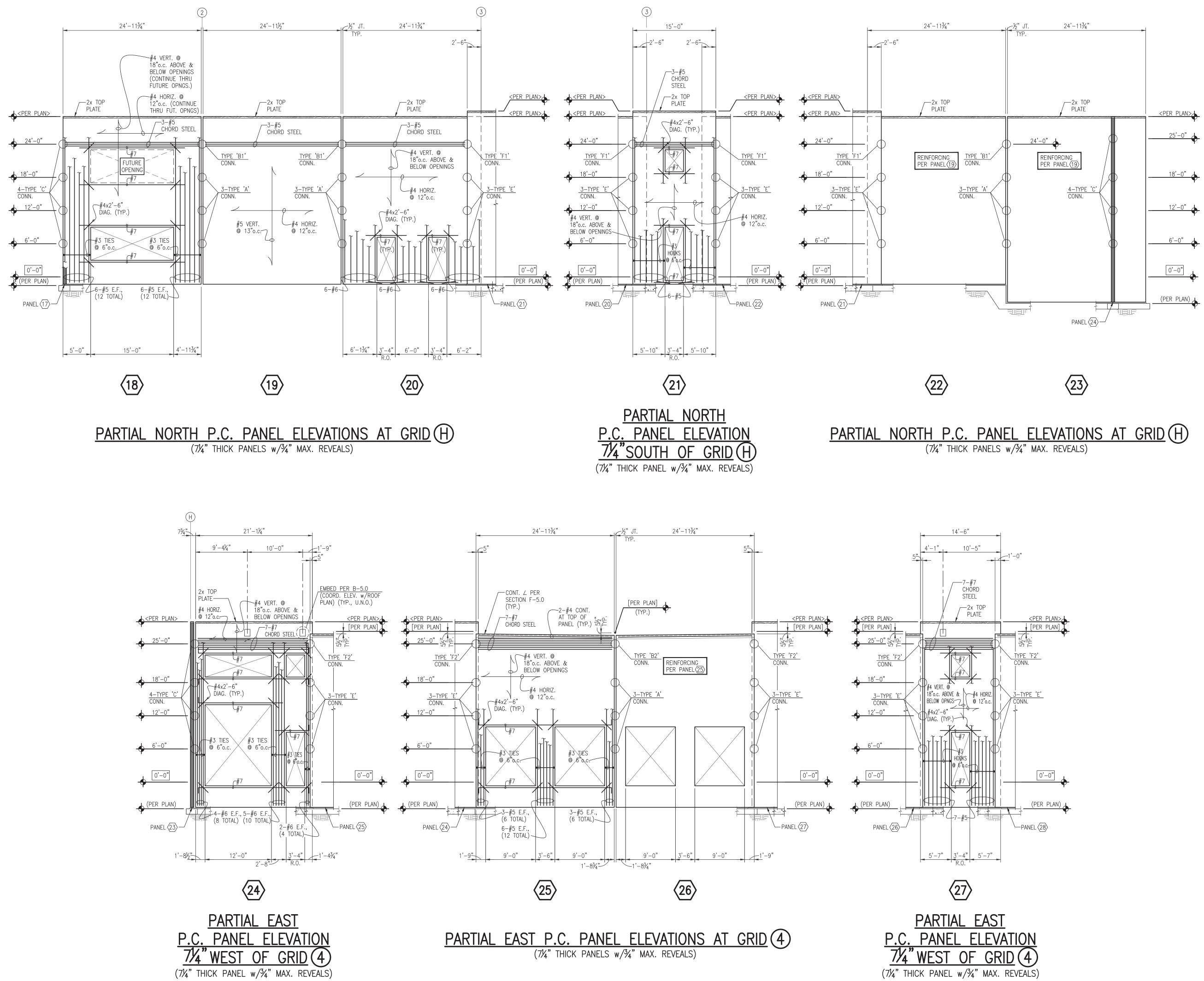
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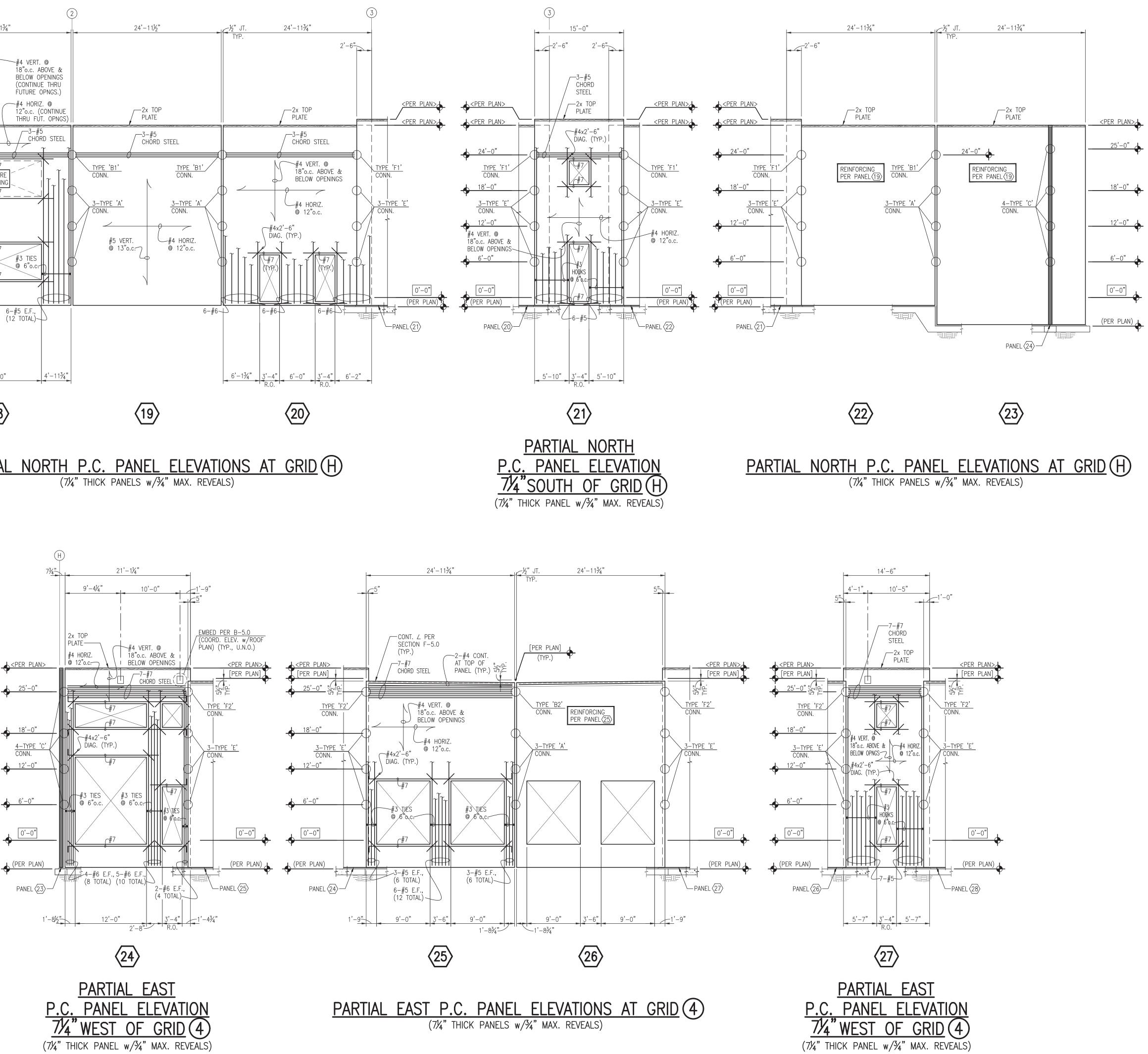
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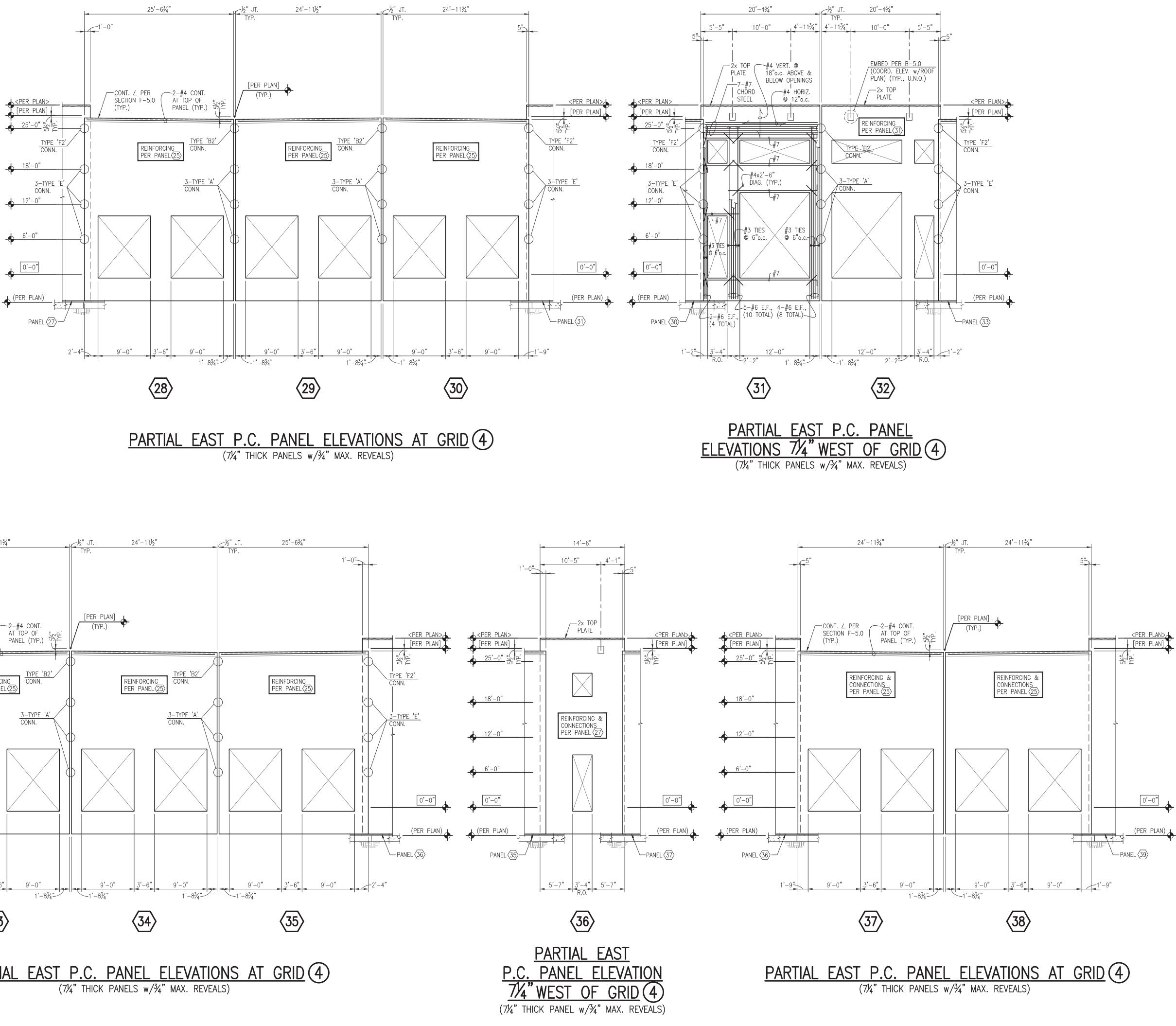
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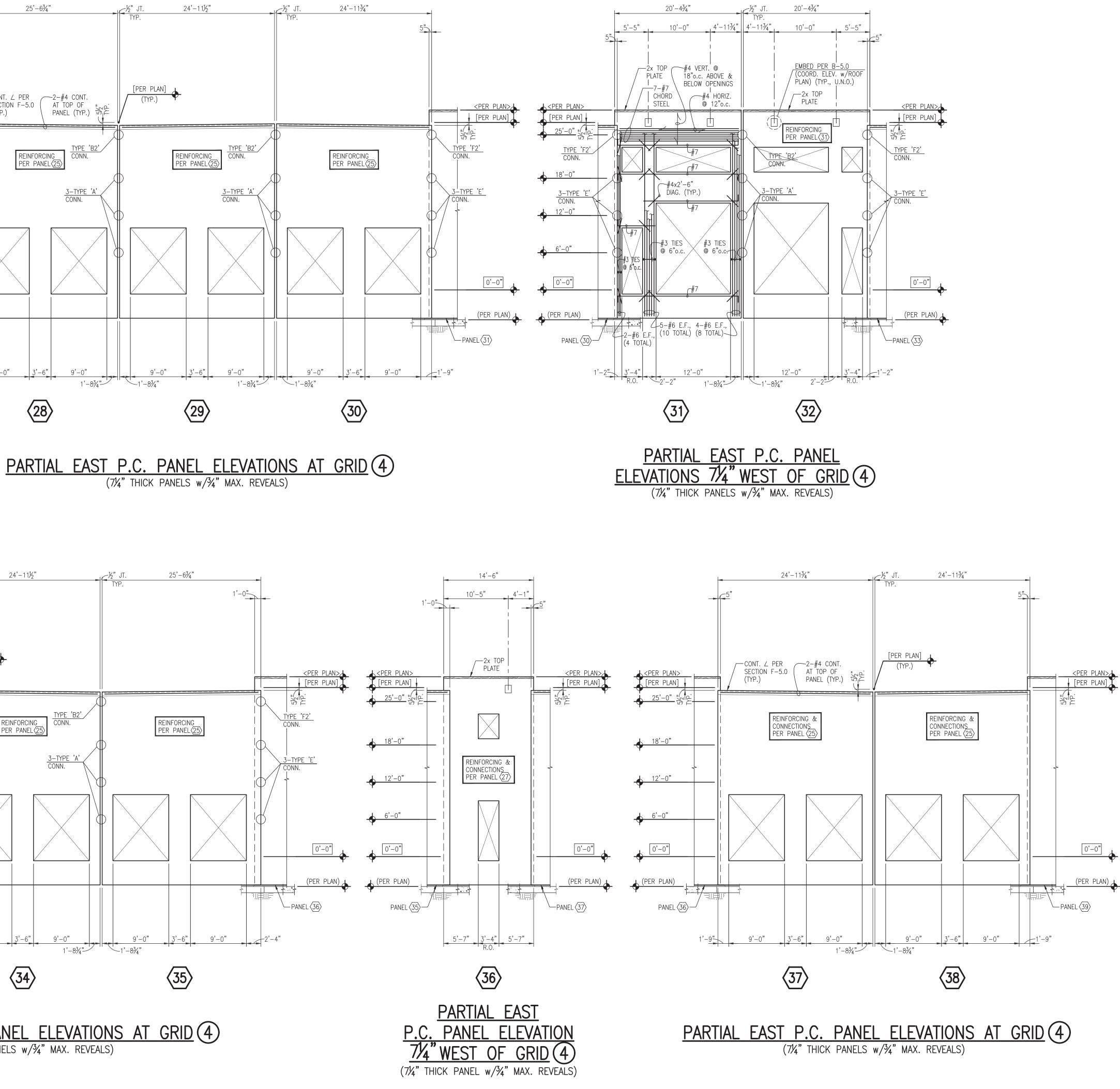
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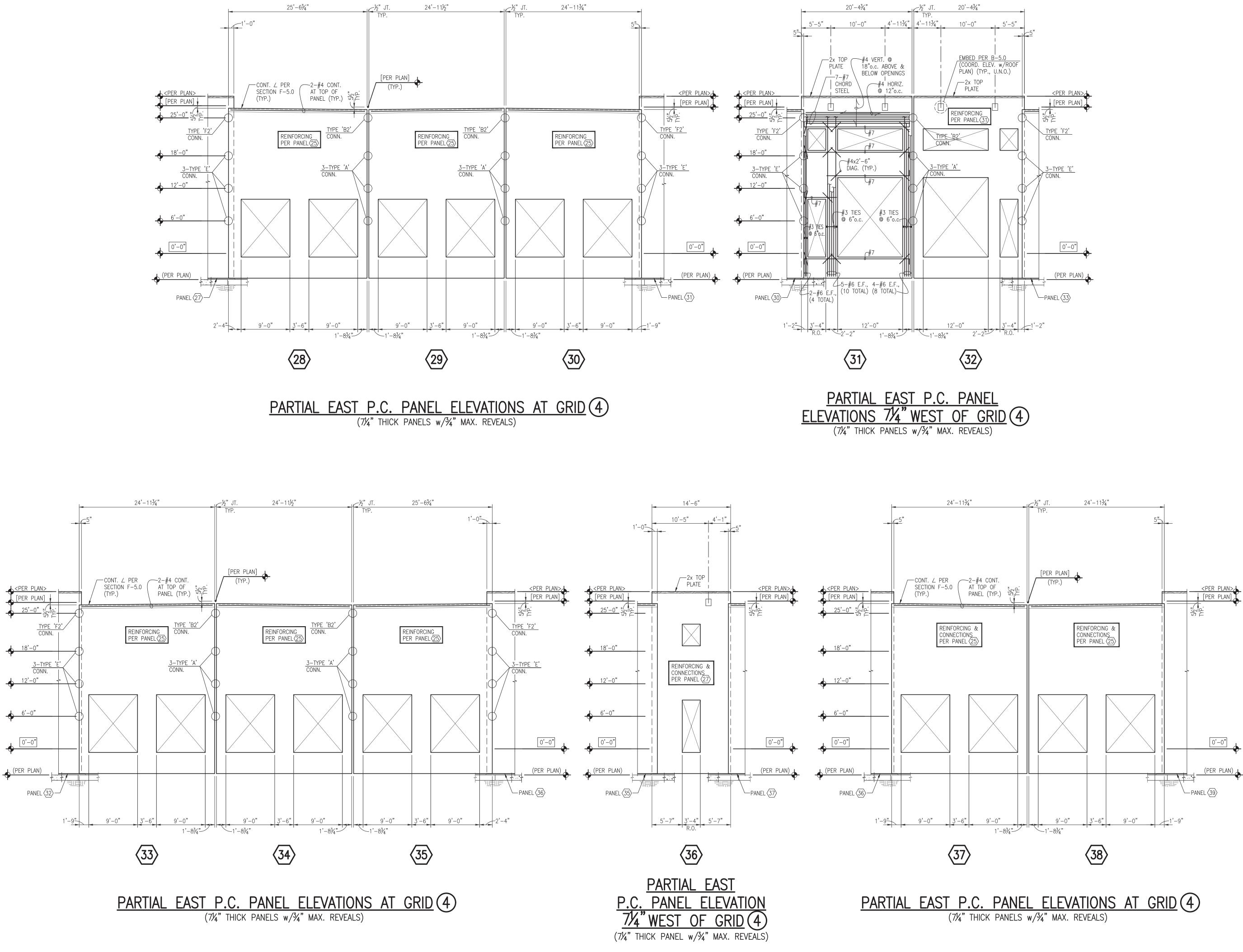
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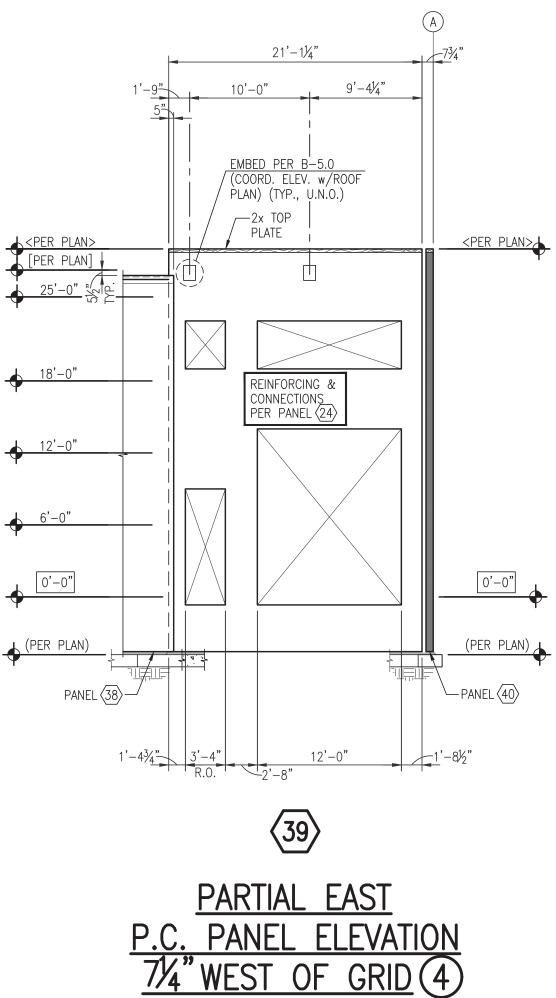
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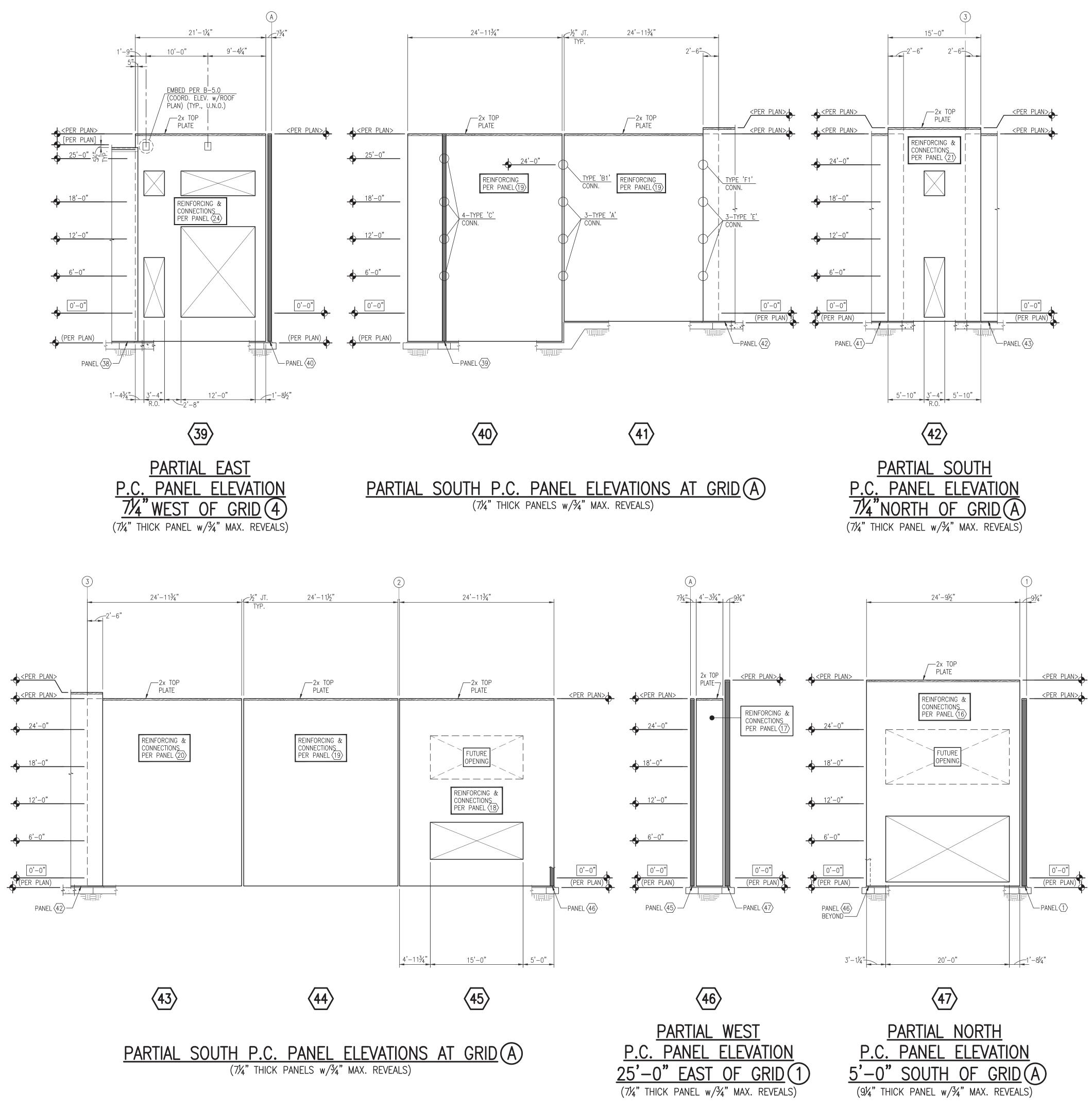
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