

DECCIO Engineering Inc.

17217 7th Avenue W. Bothell, WA. 98012 (206) 390-8374 Fax: (425) 741-8214

Spring of Life Church Commercial Building Permit

Mukilteo Permit No:

Drainage Design Report, SWPP Plans & Drainage Plans

Property Location:

4711 116th Street SW Everett, Washington



February 23, 2022

Prepared for: Spring Of Life Church

Spring of Life Church, WA STORMWATER DRAINAGE REPORT

TABLE OF CONTENTS

Introduction	1
Section 1: Pr	oject Overview
Section 2: Ex	xisting Site Conditions
	ffsite Analysis Analysis and Mitigation6
Section 4:	Minimum Requirements9
Section 5:	Stormwater Control Plan10
Section 6:	Pollution Prevention Plan
Section 7:	Special Reports and Studies
Section 8:	Other Permits
Section 9:	Operations and Maintenance

List of Figures:

Figure 1: Vicinity Map Figure 2: Site Assessment and Summary Figure 3: Site Developed Downstream Analysis and Sub-Basin Map Soils Management Form Soils Option 3

Appendix A:

Pipe Conveyance Charts WWHM12 Results

Introduction

This report has been prepared at the request of the Spring of Life Church in support of a building and parking addition located at 4711 116th Street SW Mukilteo Washington. (Parcel #00788400000500 (Refer to **Site Location and Vicinity Map**).

This report addresses the drainage report contents and organization of MR 1 to MR 9 per DOE 2014 Stormwater Management Manual as adopted by the City of Mukilteo. Specifically we have designed the drainage using the *LID applications as outlined in Volume 1, Chapter 2, Table 2.5.1 of the* DOE 2014 Stormwater Management Manual.

Section 1: Project Overview

The project consists of the construction of a building addition and added parking to an existing church site. The existing site has a building and parking which will remain. There are no known sensitive areas on or adjacent to the site.

Once developed, the project will add a total of 19,764 sf of new impervious area consisting 6,732 sf for the new building, 10,782 sf for the additional parking area and 2,500 sf of new sidewalks.

Frontage improvements are required on 116th Street SW in the form of 5-ft sidewalks and a landscape strip along with a 30-ft wide landscape buffer behind the sidewalks. The existing curb and gutter will remain

The site proposes the use of a detention vault to address the runoff from the site. Water quality for the PGHS area will be provided by a StormFilter catch basin. Reverse slopes on the new sidewalks draining back into the 30-ft wide planters will address the runoff from the new sidewalks

Minimum Requirements:

The WWHM12 program was used to size the proposed detention vault. There are no proposed deviations or any site conditions which would affect the site design.

Conditions of Approval Summary

The City will issue the Conditions of approval once the project has been reviewed and approved

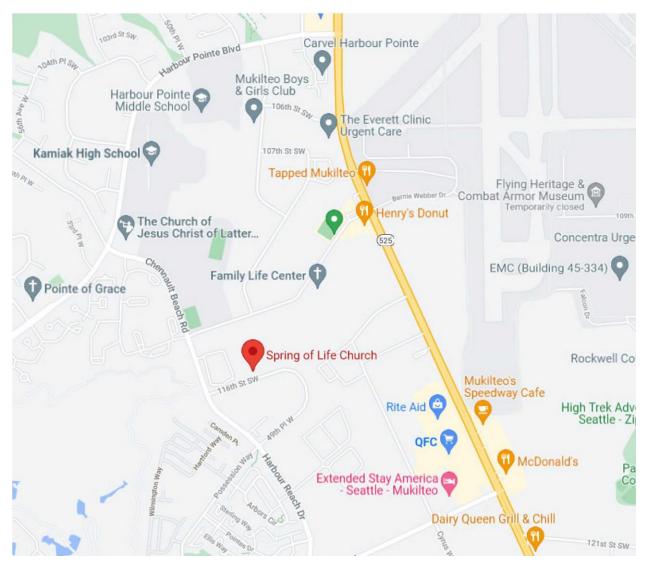
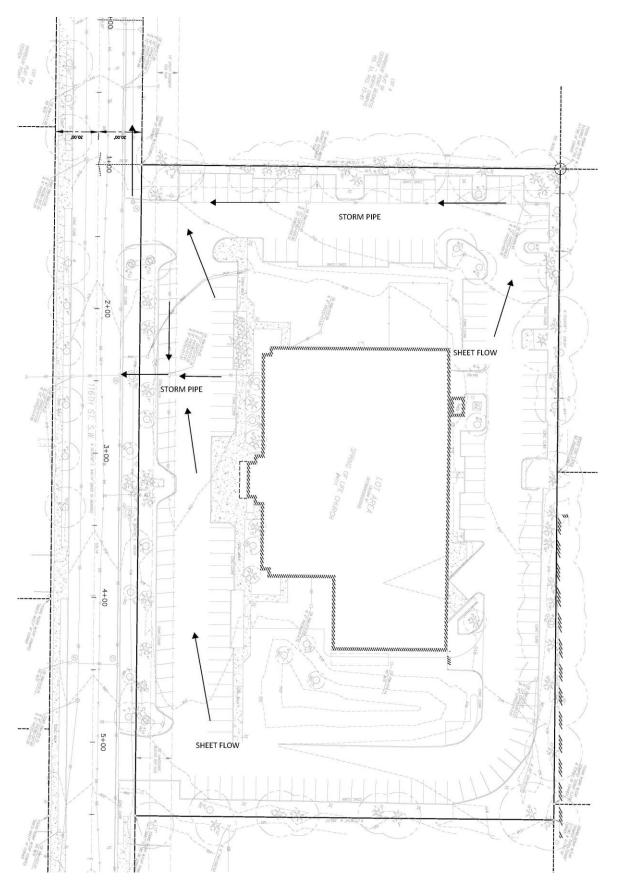


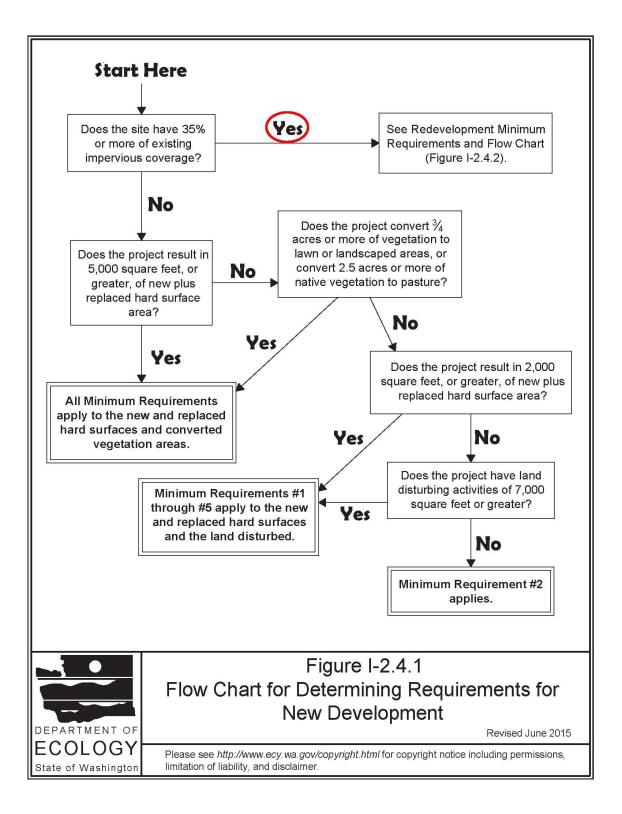
Figure 1: Vicinity Map

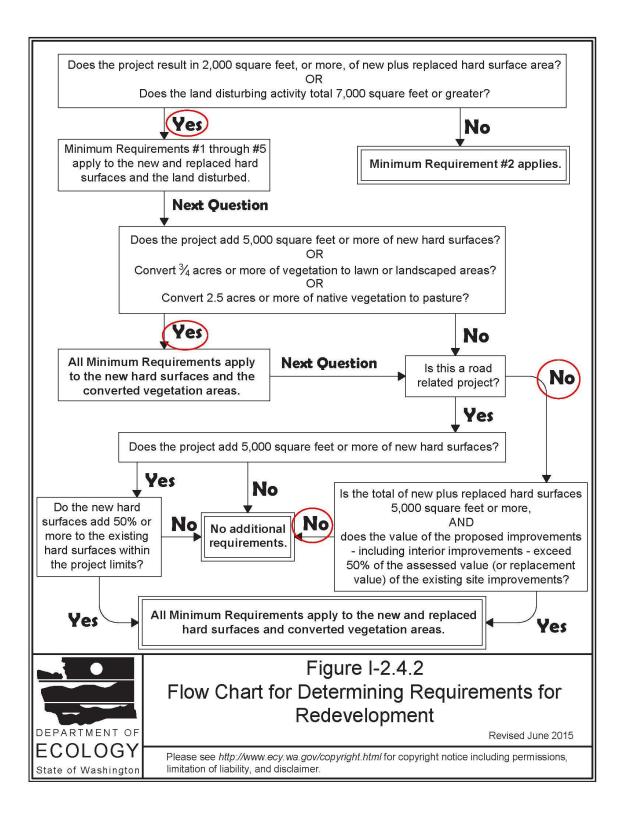
Section 2: Existing Site and Basin Assessment

The project consists of the construction of a building addition and added parking to an existing church site. The existing site has a building and parking which will remain. There are no known sensitive areas on or adjacent to the site. The site itself slopes to the south and west with slopes around 2 to 8-percent. Currently all drainage is intercepted by the storm pipe system in 116th Street SW where it flows west and discharges to a detention pond located on a golf course. Based on the site contours there is no off-site runoff draining onto the property. The area of disturbance covers only the proposed site improvements.









Section: 3 **Offsite Analysis Report**

OFF SITE ANALYSIS AND MITIGATION

An Off Site Analysis and Mitigation Study meeting requirements outlined in Section 2.5.1 was performed on November 20, 2021. The weather was overcast and cold at the time of the field inspection. See Downstream Analysis Map.

TASK I: PROJECT OVERVIEW & STUDY AREA DEFINITION

The proposed project is located on the north side of 116th St SW

Existing Site Conditions: The existing pervious cover is landscaping.

The site itself slopes to the south and west with slopes around 2 to 8-precent.

TASK 2: REVIEW OF AVAILABLE INFORMATION ON STUDY AREA **Resource Review:**

The following is a description of the resources that were reviewed for the preparation of this Level 1 Drainage Study:

A. Basin Recognizance Summary: Enclosed within are downstream and upstream basin area map which clearly define the flow pass and the drainage basins related to this project. See ("Downstream Analysis Map")

	-	and Predicted Problems onstrictions in the existing drainage system.
0	On-site:	No evidence of any problem.
0	Off-site:	Streams
		(no evidence of capacity or any other problems)
Overt	opping, Scourin	g, Bank, Sloughing of Sedimentation
0	On-site:	No evidence of any problem.
0	Off-site:	Streams None Noted
		Catch basins None Noted
<u>Flood</u>	ling etc.	
0	None Noted	

Significant Destruction of Aquatic Habitat or Organisms None Noted \bigcirc

B. Adopted Basin Plans: Puget Sound Basin

- C. Floodplain/Floodway (FEMA Maps): Does not appear to be located within the flood plain of the stream as determined by the FEMA maps.
- D. Other Off-Site Analysis Issues: None Noted

TASK 3 & 4: FIELD INSPECTION AND DESCRIPTION OF DRAINAGE SYSTEM WITH EXISTING AND PREDICTED PROBLEMS

A Level 1 drainage analysis was performed for the site on November 20, 2021 to determine any preexisting drainage problems downstream. The weather was overcast and cold. The total distance covered was over a mile downstream. (**Downstream Analysis Map**)

Level 1 Downstream Drainage Analysis:

Section 1: Upstream Drainage Analysis:

As mentioned, there is no upstream area drainage onto the site and therefore the drainage is limited to the site itself.

A review of the upstream area indicated that there were no indications of capacity problems observed in the conveyance systems upstream of the site.

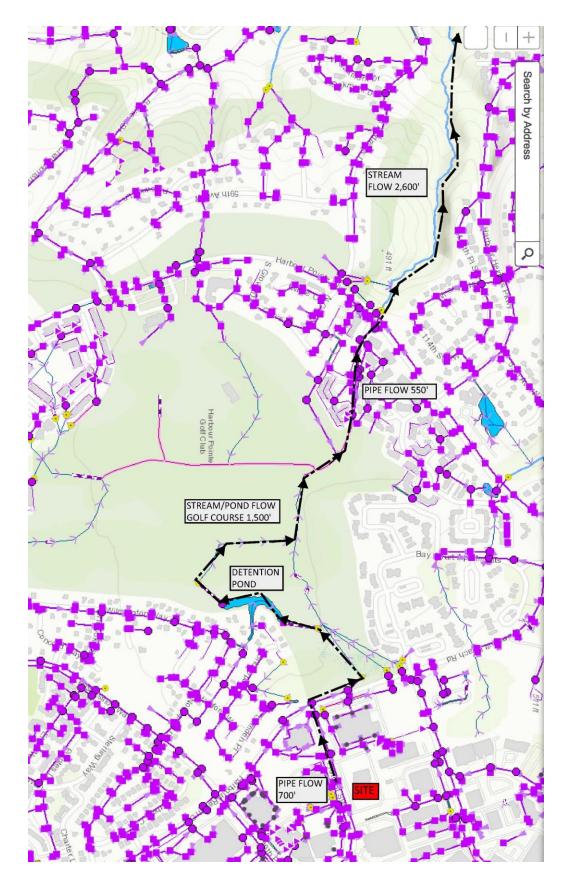
Section 2: 116th St SW to Golf Course to Stream

The drainage from the site discharge out to the street where it flows west via 12-inch storm pipes for a distance of 700-ft discharging to a stream located on the golf course. From here the stream flows west and into a detention pond located in the golf course. The runoff from the detention pond continues west via a stream to a subdivision where it flows via a storm pipes and catch basin system flowing across Harbor Point Blvd and discharging into a stream. This stream then flows west for a distance of 2,600 ft before discharging into Puget Sound.

The drainage system consisted of storm piping and stream channels. There appear to be no issues with the stream channel. However, nearly all of the downstream drainage system was on private property and not accessible to inspection.

Conclusions:

Since the project proposes detention for the site improvements, the project should have minimal impacts downstream.



Downstream Analysis Map

Section 4: Minimum Requirements

- Minimum Requirement #1: Preparation of Stormwater Plans
 - The civil plans and Section 5 of this report addresses the preparation of the stormwater plans required for this project.
- Minimum Requirement #2: Construction Stormwater Pollution Prevention Narrative.
 Section 6 of this report addresses all 13-elements of a SWPP Plan
- Minimum Requirement #3: Source Control of Pollution
 - The project does not fall under the "High Use Sites" covering commercial or industrial sites. Section 6 of this report covers further details for Source Control of Pollution.
- Minimum Requirement #4: Preservation of Natural Drainage Systems and Outfalls
 - All runoff currently leaves the site along the south-west end of the property flowing into 116th Street SW and the proposed discharge location is also at the same location, thereby maintaining the natural discharge location.
- Minimum Requirement #5: On-Site Stormwater Management BMP's
 - \circ $\,$ The project is required to use LID methods in the form of detention
- Minimum Requirement #6: Runoff Treatment
 - The site will provide water quality treatment in the form of a Contech StormFilter for the PGHS areas.
- **Minimum Requirement #7**: Flow Control
 - The detention volumes were sized using the WWHM12 program as covered under Section 5.
- **Minimum Requirement #8:** Wetlands Protection
 - \circ $\;$ There are no know sensitive areas located on or adjacent to the site.
- Minimum Requirement #9: Operations and Maintenance.
 - Section 9 of the report covers the Operations and Maintenance of the project. Which covers the StormFilter, Detention Vault and storm systems.

Section: 5 Stormwater Control Plan

MR-7 Flow Control Calculations:

Under Volume 1, Chapter 3.1.7 of the 2014 DOE-SWM Manual, the site was analyzed for detention and water quality. The project proposes to mitigate the site development improvements by providing on-site detention in the form of a vault with a StormFilter for water quality.

Per previous conversations with the City, the detention system will intercept the runoff from the proposed new parking area and the new building where it will be detained and released at the predeveloped rates.

Reverse slopes on the new sidewalks draining back into the 30-ft wide planters will address the runoff from the new sidewalks

The **WWHM Ver. 12 Hydraulic Simulation Model** was used to calculate the pre-developed and developed flows for the Hydrographs for the 2-year and 50-year, 24-hour duration design storm events for the existing and developed conditions. The hydrographs were generated using the following information:

WWHM12 Input Information

- Regional Gage Station: Everett
- Precip Scale: 1.00Soils Type: Till

Existing Site Hydrology

The existing area for the proposed site improvements is required to be forested. Therefore, the model requires a Previous Land Use to be Till, Forest. The **WWHM12** runoff was calculated from the existing forested conditions to determine the total allowable release rate from the Existing Site. (See Figure 2A: "Existing Site Conditions")

WWHM Input: On Site Area: Existing Sub-Basin

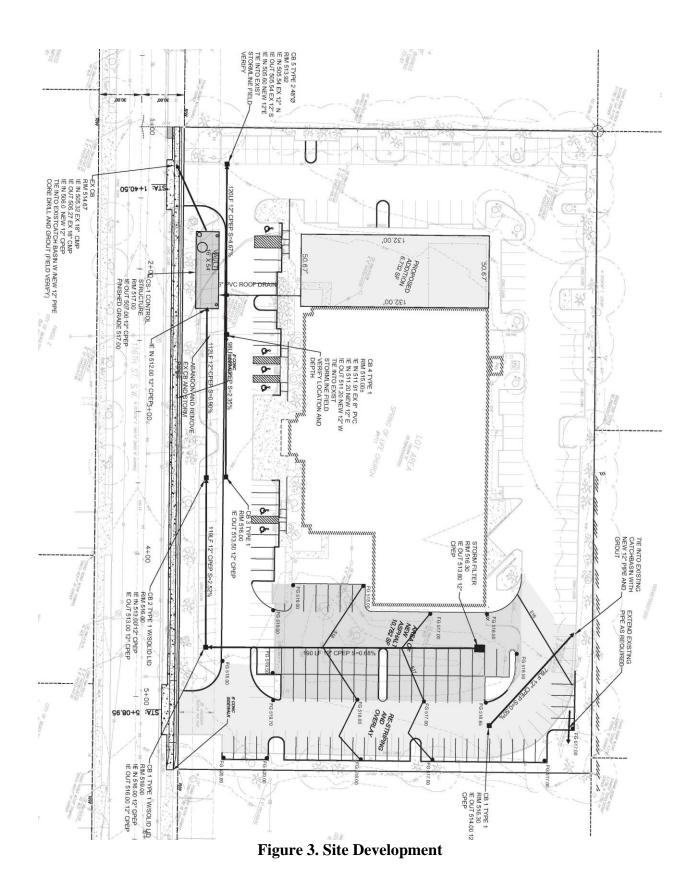
New Building Area: New Pavement Area: Total Site Area to be developed: Cover: Till, Forest, 6,732 sf <u>10,782 sf</u> 17,514 sf or 0.41 acres

Developed Site Hydrology

Once developed, the project will add a total of 17,514 sf of new impervious area consisting 6,732 sf for the new building, 10,782 sf for the additional parking area. These areas will be detained in the new detention vault.

Impervious On Site Area: Developed Sub-Basin		
New Building Area:	6,732 sf	
New Pavement Area:	<u>10,782 sf</u>	
Total Site Area to be developed:	17,514 sf or 0.41 acres	

Full Drainage Report Page 11





Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
1	Alderwood gravelly sandy loam, 0 to 8 percent slopes	136.0	77.5%
5	Alderwood-Urban land complex, 2 to 8 percent slopes	25.4	14.5%

Results of WWHM12 Computer Analysis:

	Storm Event	Exist. Site	Mitigated Site
٠	2-year, return period:	0.01355 cfs	0.00833 cfs
٠	50-year, return period:	0.03770 cfs	0.03459 cfs
•	100-year, return period:	0.04363 cfs	0.04353 cfs

Only the runoff the new site improvements will be detained and released at the required predeveloped rates See **Appendix B** for WWHM12 calculation results.

Maintenance Access: The paved parking area adjacent to the control structure will provide the required maintenance access.

Spring of Life February 23, 2022

Detention Calculation Results:

The stormwater run-off detained will provide flow control in the storage vault located at the south west corner of the property.

Results From WWHM12 Calculations:

Retention/Detention Facility

\triangleright	Type Of Facility:	Detention Vault
\triangleright	Side Slopes:	Vert Conc Walls
\triangleright	Vault Bottom Width:	16.9-ft
\triangleright	Vault Bottom Length:	50.8 ft
\triangleright	Vault Bottom Area:	859 sf
\triangleright	Effective Live Storage Depth:	7.0 ft
\triangleright	Live Storage Volume Required:	6,010 cuft
\triangleright	Live Storage Volume Provided:	6,049 cu-ft (Vault 54' x 16'x7')
\triangleright	Factor of Safety Req'd:	N/A
\triangleright	Water Quality Volume Provided	l: StormFilter
\triangleright	Riser Head:	7.0 ft
\triangleright	Riser Diameter:	12.00 inches
\succ	Number of Orifices:	3

<u>Orifices #</u>	<u>Height</u>	Diameter
1	0.00 ft	0.48 inches
2	4.11 ft	0.57 inches
3	5.24 ft	0.48 inches
Over Flow 12" Dia.	6.0 ft	12.00 inches

The results of the WWHM12 calculations are included in Appendix A

New Sidewalks: The project will provide reverse slopes on the new sidewalk draining back into the on-site planter areas to address the runoff

CONVEYANCE SYSTEM ANALYSIS AND DESIGN

The following conveyance capacity calculations for the on-site systems were calculated using the 100year developed flow results from the WWHM12 Method storm calculations with 15-minute time steps. The pipe from the parking to the vault was considered the most critical section of pipe with a slope of 0.50 %. (See **Appendix A**)

Design results:

<u>Storm Event</u>	Dev. On S	<u>Site</u>
100-year, return period:	0.45 cfs	:Developed flows for 12-inch pipe from parking area to the detention vault, is used for conveyance design.

PIPE CAPACITY: ROADWAY

The **12- CPEP pipe** from the **parking** was sized to handle the runoff from the 100-year storm. (See **Appendix A** "Pipe Conveyance Charts")

Using D.O.T. Chart 35 "De	esign Charts For Open Chan	nel Flow":		
12" CPEP Pipe	Slope:	0.50 % Min	imum Slope,	
Mar	nnings:	n = .012		
100-Yea	100-Year Dev. Flows: 0.45 cfs Design Flow			
Capacity Results:				
12" Pipe Capacity:	= 3.00 cfs (flowing full)	> 0.45 cfs required		
Velocity:	= 3.25 fps	> 3.00 fps required		
Therefore the 12" CPEP i	is adequate			

12-INCH VAULT STANDPIPE OVERFLOWS:

The **12- Stand pipes** used as a **vertical over flows** for the vault were sized to handle the runoff from the 100-year developed storm. (See **Appendix A** "Figure III-2.38 "Riser Inflows Curves")

Vault #1:

Peak Stage above overflow:	0.5- feet from overflow	to top of vault wall
Capacity Required: 0.45 cfs	Capacity Provided:	3.80 cfs
Therefore, overflow o	k	

WATER QUALITY:

Landscaping BMP T5.13 "Post Construction Soil Quality and Depth"

The top soils will be stockpile on-site and reused per "*Implementation Options #3* per Volume V, Chapter 5, BMP T5.13 "Post Construction Soil Quality and Depth" which requires "*Stockpile existing top soils during grading and replace it prior to planting*..." In addition, the soils will be required to be tested for organic compliance. (See work sheets on following pages).

Parking Water Quality: Water quality will be provided by installing a "StormFilter" Catch Basin structure from Contech Engineered Solutions. The water quality structure is upstream of the vault and was sized to pick up the runoff from all the PGHS areas including the driveway/parking areas. "TM" treating flows from PGHS areas equal to the flows of the 6-month developed design storm.

The following information was used to determine the **Mass Loading** for the final design of the filter (See design results on the next page)

Linear Storm Filter Design:

•	Total contributing area:	0.24 acres
•	PGHS area draining to the StormFilter:	0.24 acres
•	Water Quality Flow Rate:	0.027 cfs
•	Peak Hydraulic Flow Rate:	0.29 cfs
•	Height of sediment storage in detention vault:	0.5- ft



Prepared by Richard Deccio P.E. on February 28, 2022

Spring Of Life – Stormwater Treatment System

Information provided:

- Total contributing area = 0.24ac
- Impervious area = 0.24ac
- Water quality flow = 0.027cfs
- Peak hydraulic flow rate = 0.29cfs
- Presiding agency = Mukilteo

Assumptions:

- Media = ZPG cartridges
- Cartridge flow rate = 11.25 gpm
- Drop required from rim to outlet = 3.05' minimum

Size and cost estimates:

The StormFilter is a flow-based system, and is therefore sized by calculating the peak water quality flow rate associated with the design storm. The water quality flow rate was calculated by the consulting engineer using WWHM12 program

©2012 Contech Engineered Solutions LLC www.conteches.com

11835 NE Glenn Widing Dr., Portland OR 97220 Toll-free: 800.548.4667 Fax: 800.561.1271

Page 1 of 1 TS-P027

inte Designation: Date: 2/28/22 County or Independent City: Mukilteo Design Engineer: RAD itate: WA WA WA	E			
County or Independent City: Mukilteo Vea State: VVA Flow Based Data: Peak Design Flow (cfs) Annual Rainfall (inches) Total Drainage Area, A (ac) Post Development Impervious Area, A, (ac) Post Development Impervious Area, A, (ac) Pervious Area, A, (ac) % Impervious Runoff Coefficient, Rc Flow Based Filter Sizing: Flow Based Filter Si	Project Name:	Spring of Life Church		
State: WA Flow Based Data: Peak Design Flow (cfs) 0.29 Water Quality Flow (cfs) 0.027 Annual Rainfall (Inches) 36 Total Drainage Area, A (ac) 0.24 Post Development Impervious Area, A, (ac) 0.24 Pervious Area, A, (ac) 0.000 % Impervious 100% Runoff Coefficient, Rc 0.95	Site Designation:		Date:	2/28/22
Flow Based Data: Peak Design Flow (cfs) 0.29 Water Quality Flow (cfs) 0.0027 Annual Rainfall (inches) 36 Total Drainage Area, A (ac) 0.24 Pervious Area, A, (ac) 0.24 Pervious Area, A, (ac) 0.24 Pervious Area, A, (ac) 0.00 % Impervious 100% Runoff Coefficient, Rc 0.95 Flow Based Filter Sizing: 5tormFilter Filter Type StormFilter Structure Type Catchbasin (Steel) Cartridge Height 27" Media Type 2PG Cartridge Flow Rate, gpm/sf 1.00 gpm/sf Gartridges Required 1 Recommended Model SFCB1	County or Independent City:	Mukilteo	Design Engineer:	RAD
Peak Design Flow (cfs) Water Quality Flow (cfs) 0.29 Annual Rainfall (inches) 36 Total Drainage Area, A (ac) 0.24 Post Development Impervious Area, A, (ac) 0.24 Pervious Area, A _P (ac) 0.000 % Impervious 100% Runoff Coefficient, Rc 0.95 Flow Based Filter Sizing: 0.95 Filter Type StormFilter Structure Type Catchbasin (Steel) Cartridge Height 27" Media Type 2PG Cartridges Required 1 Recommended Model SFCB1	State:	WA		
Water Quality Flow (cfs) 0.027 Annual Rainfall (inches) 36 Total Drainage Area, A (ac) 0.24 Post Development Impervious Area, A, (ac) 0.24 Pervious Area, A, (ac) 0.00 % Impervious 100% Runoff Coefficient, Rc 0.95 Flow Based Filter Sizing: StormFilter Structure Type Catchbasin (Steel) Cartridge Height 27" Media Type 2PG Cartridges Required 1.00 gpm/sf Recommended Model SFCB1	Flow Based Data:			
Annual Rainfall (inches) 36 Total Drainage Area, A (ac) 0.24 Post Development Impervious Area, A, (ac) 0.24 Pervious Area, A, (ac) 0.00 % Impervious 100% Runoff Coefficient, Rc 0.95 Flow Based Filter Sizing: 0.95 Filter Type StormFilter Structure Type Catchbasin (Steel) Cartridge Height 27" Media Type ZPG Cartridges Required 1 Recommended Model SFCB1	Peak Design Flow (cfs)			0.29
Total Drainage Area, A (ac) 0.24 Post Development Impervious Area, A, (ac) 0.24 Pervious Area, A, (ac) 0.00 % Impervious 100% Runoff Coefficient, Rc 0.95 Flow Based Filter Sizing: 0.95 Filter Type StormFilter Structure Type Catchbasin (Steel) Cartridge Height 27" Media Type ZPG Cartridges Required 1 Recommended Model SFCB1	Water Quality Flow (cfs)			0.027
Post Development Impervious Area, A, (ac) Pervious Area, A, (ac) 0.24 Pervious Area, A, (ac) 0.00 % Impervious Runoff Coefficient, Rc 100% StormFilter 0.95	Annual Rainfall (inches)			36
Pervious Area, A _P (ac) 0.00 % Impervious 100% Runoff Coefficient, Rc 0.95 Flow Based Filter Sizing: Filter Type StormFilter Structure Type Catchbasin (Steel) Cartridge Height 27" Media Type ZPPG Cartridge Flow Rate, gpm/sf 1.00 gpm/sf Cartridges Required 1 Recommended Model SFCB1	Total Drainage Area, A (ac)			0.24
% Impervious 100% Runoff Coefficient, Rc 0.95 Flow Based Filter Sizing: 1 Filter Type StormFilter Structure Type Catchbasin (Steel) Structure Type 27" Media Type 2PG Cartridge Height 1.00 gpm/sf Gartridge Required 1 Recommended Model SFCB1	Post Development Impervious Area	, A ₁ (ac)		0.24
Runoff Coefficient, Rc 0.95 Flow Based Filter Sizing: StormFilter Filter Type StormFilter Structure Type Catchbasin (Steel) Catridge Height 27" Media Type ZPG Catridge Flow Rate, gpm/sf 1.00 gpm/sf Catridges Required 1 Recommended Model SFCB1	Pervious Area, A _P (ac)			0.00
Runoff Coefficient, Rc 0.95 Flow Based Filter Sizing: StormFilter Filter Type StormFilter Structure Type Catchbasin (Steel) Catridge Height 27" Media Type ZPG Catridge Flow Rate, gpm/sf 1.00 gpm/sf Catridges Required 1 Recommended Model SFCB1				
Flow Based Filter Sizing: Filter Type StormFilter Structure Type Catchbasin (Steel) Catridge Height 27" Media Type 2PG Catridge Flow Rate, gpm/sf 1.00 gpm/sf Catridges Required 1 Recommended Model SFCB1	% Impervious			100%
Filter Type StormFilter Structure Type Catchbasin (Steel) Cartridge Height 27" Media Type ZPG Cartridge Flow Rate, gpm/sf 1.00 gpm/sf Cartridges Required 1 Recommended Model SFCB1	Runoff Coefficient, Rc			0.95
Structure Type Catchbasin (Steel) Cartridge Height Cartridge Flow Rate, gpm/sf Cartridges Required Recommended Model SFCB1	Flow Based Filter Sizing:			
Cartridge Height 27" Media Type 2PG Cartridge Flow Rate, gpm/sf 1.00 gpm/sf Cartridges Required 1 Recommended Model SFCB1	Filter Type		Sto	ormFilter
Media Type ZPG Cartridge Flow Rate, gpm/sf 1.00 gpm/sf Cartridges Required 1 Recommended Model SFCB1	Structure Type		Catch	
Cartridge Flow Rate, gpm/sf Cartridges Required Recommended Model				
Cartridges Required 1 Recommended Model SFCB1	Media Type			
Recommended Model SFCB1	Cartridge Flow Rate, gpm/sf		1.0	0 gpm/sf
	Cartridges Required			1
Mavimum Water Quality Flavy	Recommended Model			SFCB1

Section 6: SWPP Plan Narrative

The proposed project will require only minimal erosion and sedimentation control measures during construction. Clearing limits will be set in the field and basically limited to the new site improvments. Silt fences will be installed on the down gradient side of the site. A rock construction entrance pad, is required to be installed off of the main road and temporary seeding of the site will take place upon reaching the final sub-grade level. Any disturbed areas will be seeded and mulched to prevent erosion.

Pollution Source Control

Once site construction is complete, some small amounts of oils and grease will be present do to the daily traffic. In addition, small amounts of silt and dirt will be present.

Critical areas: None on site.

Source of water for erosion: Rainfall hydrology is the only source of runoff.

Measures proposed to prevent/minimize erosion: With summertime measures such as silt fence, hydro-mulching and the use of straw bales as required, the risk of erosion can be minimized. Greater source control measures shall be taken during winter construction such as seeding, mulching or plastic sheeting. Good construction practices will prevent sediment from leaving the site.

Conclusion: Potential for significant erosion and pollution impacts on or offsite is considered low for the following reasons:

- The site disturbance will be kept to a minimum
- Landscaping of the site will take place immediately upon site reaching final grade.
- No significant source of water is present on the site outside rainfall.
- Erosion control BMPs will be employed and adjusted seasonally.

THE 13 ELEMENTS OF A CONSTRUCTION SWPPP

1. **Preserve Vegetation/Mark Clearing Limits:** The clearing limits are indicated on the plan sheet. Furthermore, clearing and grading will be limited to only areas that need to be disturbed for grading/construction of the road surface to preserve as much natural vegetation as possible. Field marking the clearing limits shall be completed prior to clearing and grubbing activities.

BMP's: Preserve Natural Vegetation (VEG)

Field Marking Clearing Limits (CL)

2. **Establish Construction Access:** Access to the construction site shall be limited to the rock construction entrance. The construction entrance shall be extended to provide access to the construction vehicle/equipment staging and employee parking areas.

BMP's: Stabilized Construction Entrance (CE)

- 3. **Control of Flow Rates:** Storm water detention: No detention is proposed for the site since the increase in volume is less minimal
- 4. **Installation of Sediment Controls:** Sediment control will be provided through a combination of filtration through the surround on-site vegetation, filter fence, straw bails,

BMP's: Silt Fence (FF) If required

5. Soils Stabilization: Temporary and permanent soil stabilization will be provided. Temporary stabilization will be provided through the application of straw and/or plastic sheeting to exposed, worked earth. From October 1 until April 30, no exposed soil may remain exposed and unworked for more than two days; after May 1, no exposed soil may remain exposed and unworked for more than seven days.

BMP's: Plastic Sheeting,

6. **Slope Protection:** Slopes shall be protected from erosion through cover and prevention of concentrated surface runoff flows.

BMP's: Plastic Sheeting,

- Protection of Permanent Drain Inlets: Inlet protection will pe provided for all catch basins. BMP's: N/A
- 8. **Stabilization of Channels and Outlets:** All channel slopes shall be constructed and protected against erosion in accordance with City E.D.D.S. BMP's: None required
- 9. **Pollutant Control:** Pollutants shall be controlled as described in the Potential Pollutants section of this SWPPP.
- 10. **Dewatering Control:** De-watering: Interception of the water table is not expected to occur, even if there is an increase in precipitation. However, should ground water flows be encountered, the flows can be directed to on site native vegetation for cleanup.

BMP's: Native vegetation (As Required)

- 11. **BMP Maintenance:** All BMP's and SWPPP elements shall be inspected daily and maintained as required.
- 12. **Project Management:** The project shall be managed in a cooperative effort by the project manager, contractor, engineer, and the City inspector. During the construction process, if unforeseen issues arise that cannot be resolved on site, construction activity (other than SWPPP maintenance) shall be halted and the City inspector and the project engineer are to be contacted and informed of the situation.

13. Protect On-Site Stormwater Management BMPs For Runoff From Roofs And Other Hard Surface

On-site stormwater management BMPs used for runoff from roofs and other hard surfaces include: full dispersion, roof downspout full infiltration or dispersion systems, perforated stubout connections, rain gardens, bioretention systems, permeable pavement, sheetflow dispersion, and concentrated flow dispersion. The areas on the site to be used for these BMPs shall be protected

from siltation and compaction during construction by sequencing the construction in a fashion to install these BMPs at the latter part of the construction grading operations, by excluding equipment from the BMPS and the associated areas, and by using the erosion and sedimentation control BMPs. BMP C102: Buffer Zone

Water Pollution Source Control

Since the project is for a residential lot, the project does not fall under the "High Use Sites" covering commercial or industrial sites.

BMP C-151: Concrete Handling (Design and Installation Specifications)

Concrete truck chutes, pumps, and internals shall be washed out only into formed areas awaiting installation of concrete or asphalt. Unused concrete remaining in the truck and pump shall be returned to the originating batch plant for recycling.

Hand tools including, but not limited to, screeds, shovels, rakes, floats, and trowels shall be washed off only into formed areas awaiting installation of concrete or asphalt.

Equipment that cannot be easily moved, such as concrete pavers, shall only be washed in areas that do not directly drain to natural or constructed stormwater conveyances.

Washdown from areas such as concrete aggregate driveways shall not drain directly to natural or constructed stormwater conveyances.

When no formed areas are available, washwater and leftover product shall be contained in a lined container. Contained concrete shall be disposed of in a manner that does not violate groundwater or surface water quality standards

Maintenance Standards

Containers shall be checked for holes in the liner daily during concrete pours and repaired the same day

Section: 7 Special Reports and Studies

• None

Section: 8 Other Permits

- Building Permit required.
- (R/W) permit required

Section: 9 Operations and Maintenance

Stormwater System Description

The stormwater system for the access road and detention/water quality vault is fairly basic and contains the following elements:

- 100 lf of inch storm pipe
- Catch basins:
- Detention Vault
- Vault control structure with orifice release flows
- StormFilter 360

The stormwater runoff from the site is intercepted by the curbs and gutter, catch basins and storm piping, where it is directed over to the detention vault for controlled release. The discharge pipe will be located at the south-west corner of the site.

Water quality treatment of the site is accomplished through the use of a StormFilter 360 water quality manhole.

Storm Filter Cartridges O&M

- A. Filter cartridges shall be delivered with the manhole. Contractor shall take appropriate action to protect the cartridges from sediment and other debris during construction. Methods for protecting the cartridges include but are not limited to:
 - 1. Remove cartridges from the manhole and store appropriately. Cartridges shall be reinstalled to operate according to 3.4 B (see below).
 - 2. Leave cartridges in the vault and plug inlet and outlet pipe to prevent stormwater from entering the vault.

The method ultimately selected shall be at Contractor's discretion and Contractor's risk

- B. Filter cartridges shall not be placed in operation until the vault is clean and the project site is clean and stabilized (construction erosion control measures no longer required). The project site includes any surface that contributes storm drainage to the StormFilter. All impermeable surfaces shall be clean and free of dirt and debris. All catch basins, manholes and pipes shall be free of dirt and sediments. Contact Contech Engineered Solutions LLC . to assist with system activation and/or inspect the system for proper installation once site is clean and stabilized.
- C. Contractor to install filter cartridges.
 - 1) Filter Cartridges With CSF Media and Slip Connector Fittings: Tape shall be cleanly and completely removed from manifold fitting openings. Spool pieces (slip fittings) shall be inserted without glue into all manifold fittings to be equipped with a filter cartridge. Filter cartridges shall be placed over the spool pieces to contact the vault floor. Plugs shall be inserted without glue in all manifold fittings not equipped with a filter cartridge.
 - 2) *Filter Cartridges with Threaded Connector Fittings:* Tape shall be cleanly and completely removed from manifold fitting openings. Threaded connectors shall be glued and inserted into all manifold fittings to be equipped with a filter cartridge. Filter cartridges shall be threaded onto the connectors until they contact the vault floor. Plugs shall be inserted without glue in all manifold fittings not equipped with a filter cartridge.
 - 3) Filter Cartridges with ¼-Turn Connector Fittings: Tape shall be cleanly and completely removed from manifold fitting openings. ¼-turn connects shall be glued and inserted into all manifold fittings to be equipped with a filter cartridge. Filter cartridges shall be turned onto the connector until they reach the hard stop on the connector – approximately ¼ revolution. Plugs shall be inserted without glue in all manifold fittings not equipped with a filter cartridge.
- PART 4 PERFORMANCE
- 4.1 Cartridge Operation

- A. Each stormwater filtration system shall contain one or more siphon actuated media filter cartridges that maintain a uniform pressure profile across the face of the filter during operation. At the design flow rate the maximum filter hydraulic loading rate is not to exceed 2.1 gallons per minute per square foot of filter surface area. Stormwater shall enter the filter cartridges through sides and shall flow through the filter media radially from the outer perimeter to the inner cartridge lumen and shall have an average contact time no less than 38 seconds.
- 4.2 Documentation of Sediment Removal
 - A. The Filtration system should have the Washington GULD certification and approval from New Jersey DEP.
- 4.3 Cartridge Sediment Loading
 - A. Filter cartridges shall be of a design that has demonstrated a minimum sediment retention capacity of 22 pounds of silty loam per cartridge in laboratory tests without a reduction in hydraulic capacity. Laboratory data shall be corroborated with field observations showing similar longevity without impact to normal hydraulic performance of the stormwater filtration system. All laboratory and field tests submitted in support of this specification must have undergone peer review.
- 4.4 Overflow
 - A. The filter system will have a baffled, non-siphoning internal overflow with a minimum of 1.0 cfs capacity.

Access Roads/Easement

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
General	Trash and Debris	Trash and debris exceeds 1 cubic foot per 1,000 square feet, i.e., trash and debris would fill up one standard size garbage can.	
	Blocked Roadway	Debris which could damage vehicle tires (glass or metal).	Roadway free of debris which could damage tires.
		Any obstructions which reduce clearance above road surface to less than 14 feet.	Roadway overhead clear to 14 feet high.
		Any obstructions restricting the access to a 10- to 12-foot width for a distance of more than 12 feet or any point restricting access to less than a 10-foot width.	Obstruction removed to allow at least a 12-foot access.
Road Surface	Settlement, Potholes, Mush Spots, Ruts	When any surface defect exceeds 6 inches in depth and 6 square feet in area. In general, any surface defect which hinders or prevents maintenance access.	Road surface uniformly smooth with no evidence of settlement, potholes, mush spots, or ruts.
	Vegetation in Road Surface	Weeds growing in the road surface that are more than 6 inches tall and less than 6 inches apart within a 400-square foot area.	Road surface free of weeds taller than 2 inches.
Shoulders and Ditches	Erosion Damage	Erosion within 1 foot of the roadway more than 8 inches wide and 6 inches deep.	Shoulder free of erosion and matching the surrounding road.
	Weeds and Brush	Weeds and brush exceed 18 inches in height or hinder maintenance access.	Weeds and brush cut to 2 inches in height or cleared in such a way as to allow maintenance access.

	NO. 3 – CLOSED DETENTION SYSTEMS (PIPES/TANKS)		
Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
Storage Area	Plugged Air Vents	One-half of the cross section of a vent is blocked at any point with debris and sediment.	Vents free of debris and sediment.
	Debris and Sediment	Accumulated sediment depth exceeds 10% of the diameter of the storage area for ½ length of storage vault or any point depth exceeds 15% of diameter. Example: 72- inch storage tank would require cleaning when sediment reaches depth of 7 inches for more than ½ length of tank.	All sediment and debris removed from storage area.
	Joints Between Tank/Pipe Section	Any crack allowing material to be transported into facility.	All joints between tank/pipe sections are sealed.
	Tank/Pipe Bent Out of Shape	Any part of tank/pipe is bent out of shape more than 10% of its design shape.	Tank/pipe repaired or replaced to design.
Manhole	Cover not in Place	Cover is missing or only partially in place. Any open manhole requires maintenance.	Manhole is closed.
	Locking Mechanism Not Working	Mechanism cannot be opened by one maintenance person with proper tools. Bolts into frame have less than ½ inch of thread (may not apply to self- locking lids).	Mechanism opens with proper tools.
	Cover Difficult to Remove	One maintenance person cannot remove lid after applying 80 lbs of lift. Intent is to keep cover from sealing off access to maintenance.	Cover can be removed and reinstalled by one maintenance person.
	Ladder Rungs Unsafe	King County Safety Office and/or maintenance person judges that ladder is unsafe due to missing rungs, misalignment, rust, or cracks.	Ladder meets design standards and allows maintenance persons safe access.
Catch Basins		See "Catch Basins" Standard No. 5	See "Catch Basins" Standard No. 5

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
Pipes	Sediment and Debris	Accumulated sediment that exceeds 20% of the diameter of the pipe.	Pipe cleaned of all sediment and debris.
	Vegetation	Vegetation that reduces free movement of water through pipes.	All vegetation removed so water flows freely through pipes.
	Damaged	Protective coating is damaged; rust is causing more than 50% deterioration to any part of pipe.	Pipe repaired or replaced.
		Any dent that decreases the cross- section area of pipe by more than 20%.	Pipe repaired or replaced.
Open Ditches	Trash and Debris	Trash and debris exceeds 1 cubic foot per 1,000 square feet of ditch and slopes.	Trash and debris cleared from ditches.
	Sediment	Accumulated sediment that exceeds 20% of the design depth.	Ditch cleaned/flushed of all sediment and debris so that matches design.
	Vegetation	Vegetation that reduces free movement of water through ditches.	Water flows freely through ditches.
Side Slopes	Erosion	Eroded damage over 2 inches deep where cause of damage is still present or where there is potential for continued erosion.	Slopes should be stabilized by using appropriate erosion control measure(s): e.g., rock reinforcement, planting of grass, compaction.

Conveyance Systems (Pipes & Ditches)

Grounds (Landscaping)

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
General	Weeds (Non- poisonous)	Weeds growing in more than 20% of the landscaped area (trees and shrubs only).	Weeds present in less than 5% of the landscaped area.
	Safety Hazard	Any presence of poison ivy or other poisonous vegetation.	No poisonous vegetation present in a landscaped area.
	Trash or Litter	Paper, can, bottles, totaling more than 1 cubic foot within a landscaped area (trees and shrubs only) of 1,000 square feet.	Area clear of litter.
Trees and Shrubs	Damage	Limbs or parts of trees or shrubs that are split or broken which affect more than 25% of the total foliage of the tree or shrub.	Trees and shrubs with less than 5% of the total foliage with split or broken limbs.
		Trees or shrubs that have been blown down or knocked over.	Trees or shrub in place free of injury.
		Trees or shrubs which are not adequately supported or are leaning over, causing exposure of the roots.	Tree or shrub in place and adequately supported; remove any dead or diseased trees.

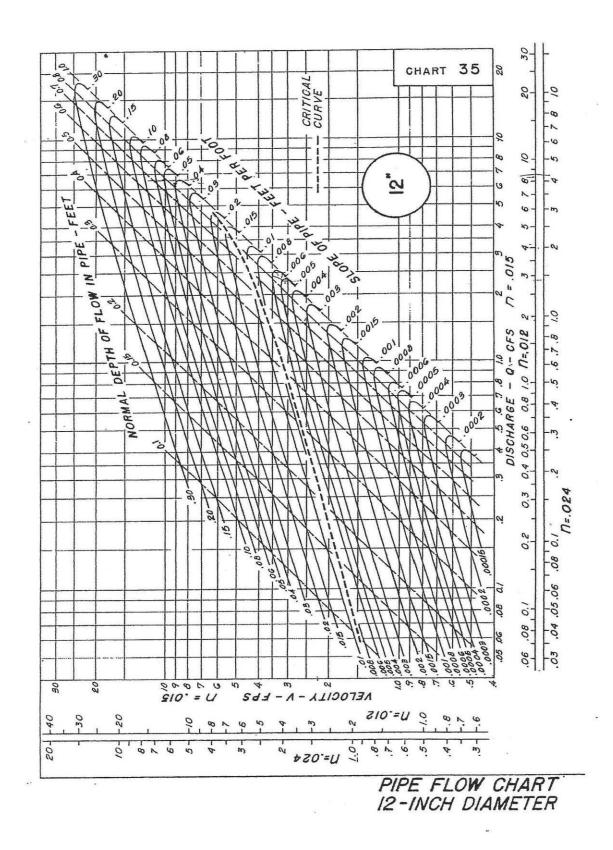
NO. 5 - CATCH BA	SINS
------------------	------

Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
General	Trash and Debris (Includes Sediment)	Trash or debris of more than ½ cubic foot which is located immediately in front of the catch basin opening or is blocking capacity of basin by more than 10%.	No trash or debris located immediately in front of catch basin opening.
		Trash or debris (in the basin) that exceeds 1/3 the depth from the bottom of basin to invert of the lowest pipe into or out of the basin.	No trash or debris in the catch basin.
		Trash or debris in any inlet or outlet pipe blocking more than 1/3 of its height.	Inlet and outlet pipes free of trash or debris.
		Dead animals or vegetation that could generate odors that would cause complaints or dangerous gases (e.g. methane)	No dead animals or vegetation present within the catch basin.
		Deposits of garbage exceeding 1 cubic foot in volume.	No condition present which would attract or support the breeding of insects or rodents.
	Structural Damage to Frame and/or Top Slab	Corner of frame extends more than $\frac{3}{4}$ inch past curb face into the street (if applicable)	Frame is even with curb.
		Top slab has holes larger than 2 square inches or cracks wider than 1/4 inch (intent is to make sure all material is running into the basin).	Top slab is free of holes and cracks.
		Frame not sitting flush on top slab, i.e. separation of more than ¾ inch of the frame from the top slab.	Frame is sitting flush on top slab.
	Cracks in Basin Walls/Bottom	Cracks wider than ½ inch and longer than 3 feet, any evidence of soil particles entering catch basin through cracks, or maintenance person judges that structure is unsound.	Basin replaced or repaired to design standards.
		Cracks wider than ½ inch and longer than 1 foot at the joint of any inlet/outlet pipe or any evidence of soil particles entering catch basin through cracks.	No cracks more than ¼ inch wide at the joint of inlet/outlet pipe.
	Settlement/Misalignme nt	Basin has settled more than 1 inch or has rotated more than 2 inches out of alignment.	Basin replaced or repaired to design standards.

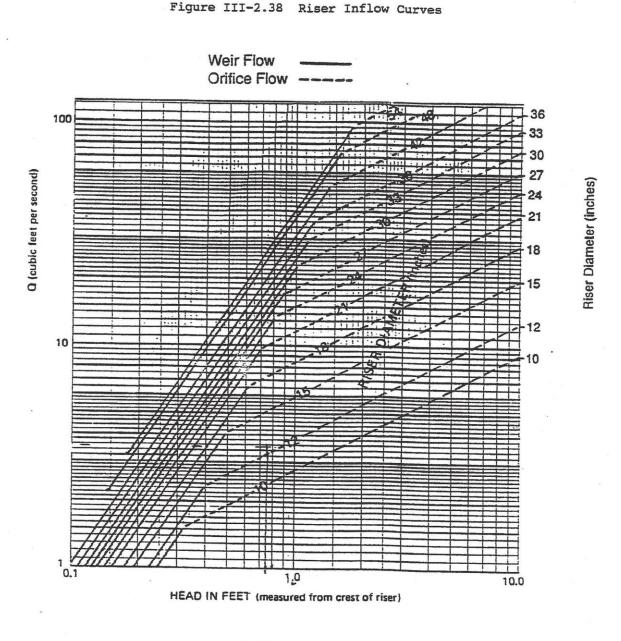
		NO. 5 – CATCH BASINS	
Maintenance Component	Defect	Conditions When Maintenance is Needed	Results Expected When Maintenance is Performed
	Fire Hazard	Presence of chemicals such as natural gas, oil, and gasoline.	No flammable chemicals present.
	Vegetation	Vegetation growing across and blocking more than 10% of the basin opening.	No vegetation blocking opening to basin.
		Vegetation growing in inlet/outlet pipe joints that is more than six inches tall and less than six inches apart.	No vegetation or root growth present.
	Pollution	Non-flammable chemicals of more than ½ cubic foot per three feet of basin length.	No pollution present other than surface film.
Catch Basin Cover	Cover Not In Place	Cover is missing or only partially in place. Any open catch basin requires maintenance.	Catch basin cover is closed.
	Locking Mechanism Not Working	Mechanism cannot be opened by one maintenance person with proper tools. Bolts into frame have less than ½ inch of thread.	Mechanism opens with proper tools.
	Cover Difficult to Remove	One maintenance person cannot remove lid after applying 80 lbs. of lift; intent is keep cover from sealing off access to maintenance.	Cover can be removed by one maintenance person.
Ladder	Ladder Rungs Unsafe	Ladder is unsafe due to missing rungs, misalignment, rust, cracks, or sharp edges.	Ladder meets design standards and allows maintenance person safe access.
Metal Grates (if applicable)		Grate with opening wider than 7/8 inch.	Grate openings meet design standards.
	Trash and Debris	Trash and debris that is blocking more than 20% of grate surface.	Grate free of trash and debris.
	Damaged or Missing	Grate missing or broken member(s) of the grate.	Grate is in place and meets design standards.

Appendix A:

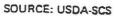
Pipe Conveyance Calculation Charts WWHM12 Method Results



Spring of Life February 23, 2022 Full Drainage Report Page 32



STORMWATER MANAGEMENT MANUAL FOR THE PUGET SOUND BASIN



Q_{wier} = 9.739 DH^{3/2}

Q_{ORIFICE} = 3.782 D²H^{1/2}

Q in cfs, D and H in feet

WWHM2012 PROJECT REPORT

General Model Information

Project Name:	default[2]
Site Name:	
Site Address:	
City:	
Report Date:	1/19/2022
Gage:	Everett
Data Start:	1948/10/01
Data End:	2009/09/30
Timestep:	15 Minute
Precip Scale:	1.000
Version Date:	2019/09/13
Version:	4.2.17

POC Thresholds

Low Flow Threshold for POC1:	50 Percent of the 2 Year
High Flow Threshold for POC1:	50 Year

Landuse Basin Data Predeveloped Land Use

Basin 1

Bypass:	No
GroundWater:	No
Pervious Land Use C, Forest, Mod	acre 0.41
Pervious Total	0.41
Impervious Land Use	acre
Impervious Total	0
Basin Total	0.41
Flowert Flower Tex	

Element Flows To: Surface Int

Interflow

Groundwater

Mitigated Land Use

Basin 1

Bypass:	No
GroundWater:	No
Pervious Land Use	acre
Pervious Total	0
Impervious Land Use ROOF TOPS FLAT DRIVEWAYS FLAT	acre 0.11 0.3
Impervious Total	0.41
Basin Total	0.41

Element Flows To:	
Surface	Interflow
Vault 1	Vault 1

Groundwater

Routing Elements Predeveloped Routing

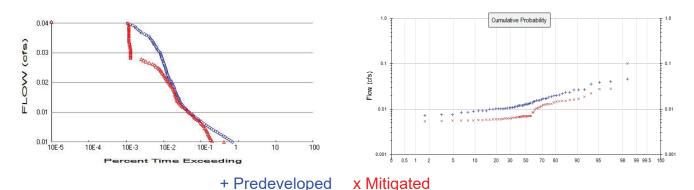
Mitigated Routing

Vault 1 Width: Length: Depth:	16.9251873393584 ft. 50.7755620180747 ft. 8 ft.
Discharge Structure Riser Height: Riser Diameter: Orifice 1 Diameter: Orifice 2 Diameter: Orifice 3 Diameter: Element Flows To: Outlet 1	7 ft. 18 in. 0.48 in. Elevation:0 ft. 0.57 in. Elevation:4.109 ft. 0.48 in. Elevation:5.242916666666669 ft. Outlet 2

Vault Hydraulic Table

Stage(feet) 0.0000 0.0889 0.1778 0.2667 0.3556 0.4444 0.5333 0.6222 0.7111 0.8000 0.8889 0.9778 1.0667 1.1556 1.2444 1.3333 1.4222 1.5111 1.6000 1.6889 1.7778 1.8667 1.9556 2.0444 2.1333 2.2222 2.3111 2.4000 2.4889 2.5778 2.6667 2.7556	Area(ac.) 0.019	Volume(ac-ft.) 0.000 0.001 0.003 0.005 0.007 0.008 0.010 0.012 0.014 0.015 0.017 0.019 0.021 0.022 0.024 0.026 0.028 0.029 0.031 0.033 0.035 0.035 0.036 0.038 0.040 0.042 0.043 0.045 0.047 0.049 0.050 0.052 0.052 0.054	Discharge(cfs 0.000 0.001 0.001 0.002 0.002 0.002 0.002 0.003 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.004 0.005 0.005 0.005 0.005 0.005 0.005 0.005 0.005	0.000 0
2.4000	0.019	0.047	0.005	0.000
2.4889	0.019	0.049	0.005	0.000
2.5778	0.019	0.050	0.005	0.000

Analysis Results POC 1



Predeveloped Landuse Totals for POC #1 Total Pervious Area: 0.41 Total Impervious Area: 0

Mitigated Landuse Totals for POC #1 Total Pervious Area: 0 Total Impervious Area: 0.41

Flow Frequency Method: Log Pearson Type III 17B

Flow Frequency Return Periods for Predeveloped. POC #1Return PeriodFlow(cfs)2 year0.013555 year0.02027710 year0.02525725 year0.03214250 year0.037701

0.043629

Flow Frequency Return Periods for Mitigated. POC #1

Return Period	Flow(cfs) `
2 year	0.008327
5 year	0.013715
10 year	0.018597
25 year	0.026637
50 year	0.034259
100 year	0.043537

Annual Peaks

100 year

Annual Peaks for Predeveloped and Mitigated. POC #1

rear	Preaevelopea	wiitigate
1949	0.009	0.007
1950	0.016	0.007
1951	0.012	0.006
1952	0.010	0.006
1953	0.009	0.006
1954	0.039	0.007
1955	0.020	0.013
1956	0.016	0.015
1957	0.019	0.011
1958	0.016	0.006

Ranked Annual Peaks

Ranked Annual Peaks for Predeveloped and Mitigated. POC #1 Rank Predeveloped Mitigated

1	0.0461	0.1002
2	0.0407	0.0280
3	0.0385	0.0273

Duration Flows

The Facility PASSED

Flow(cfs) 0.0068	Predev 14904	Mit 8806	Percentage	Pass/Fail Pass
0.0071 0.0074	13336 11907	4276 3888	32 32	Pass Pass
0.0077	10613	3788	35	Pass
0.0080	9428	3649	38	Pass
0.0083	8397	3533	42	Pass
0.0086	7490	3379	45	Pass
0.0090	6656	3292	49 54	Pass
0.0093 0.0096	5905 5217	3200 3099	54 59	Pass Pass
0.0099	4624	2969	64	Pass
0.0102	4096	2830	69	Pass
0.0105	3683	2680	72	Pass
0.0108	3266	2541	77	Pass
0.0111	2939 2627	2402 2269	81 86	Pass
0.0115 0.0118	2372	2209	88	Pass Pass
0.0121	2145	1956	91	Pass
0.0124	1948	1767	90	Pass
0.0127	1790	1600	89	Pass
0.0130	1621	1461	90	Pass
0.0133 0.0136	1471 1338	1386 1301	94 97	Pass Pass
0.0140	1230	1197	97	Pass
0.0143	1121	1096	97	Pass
0.0146	1034	1012	97	Pass
0.0149	946	929	98	Pass
0.0152 0.0155	871 812	852 785	97 96	Pass Pass
0.0158	755	709	93	Pass
0.0161	710	650	91	Pass
0.0165	671	606	90	Pass
0.0168	627	571	91	Pass
0.0171 0.0174	603 583	537 510	89 87	Pass Pass
0.0177	567	488	86	Pass
0.0180	548	478	87	Pass
0.0183	526	469	89	Pass
0.0186	502	455 444	90	Pass
0.0190 0.0193	484 468	435	91 92	Pass Pass
0.0196	456	425	93	Pass
0.0199	442	412	93	Pass
0.0202	428	398	92	Pass
0.0205 0.0208	420 409	384 366	91 89	Pass Pass
0.0208	398	344	86	Pass
0.0215	385	332	86	Pass
0.0218	374	318	85	Pass
0.0221	354	297	83	Pass
0.0224 0.0227	344 331	281 269	81 81	Pass Pass
0.0227	312	269 259	83	Pass Pass
3.0200	J.L	_00		

0.0233 0.0236 0.0240 0.0243 0.0246 0.0249 0.0252 0.0255 0.0258 0.0261 0.0265 0.0268 0.0271 0.0274 0.0277 0.0280 0.0283 0.0286 0.0290 0.0293 0.0296 0.0290 0.0293 0.0296 0.0290 0.0293 0.0296 0.0290 0.0305 0.0305 0.0305 0.0311 0.0315 0.0315 0.0324 0.0327 0.0336 0.0340 0.0343 0.0346 0.0340 0.0343 0.0346 0.0343 0.0346 0.0349 0.0355 0.0355 0.0355 0.0355 0.0355 0.0368 0.0371 0.0374 0.0374	$\begin{array}{c} 296\\ 288\\ 279\\ 273\\ 266\\ 258\\ 249\\ 245\\ 249\\ 245\\ 225\\ 219\\ 214\\ 203\\ 197\\ 188\\ 185\\ 179\\ 175\\ 168\\ 157\\ 145\\ 130\\ 123\\ 117\\ 103\\ 97\\ 91\\ 86\\ 74\\ 655\\ 52\\ 47\\ 39\\ 38\\ 32\\ 27\\ 22\end{array}$	$\begin{array}{c} 246\\ 235\\ 225\\ 215\\ 203\\ 195\\ 183\\ 173\\ 162\\ 151\\ 133\\ 121\\ 107\\ 88\\ 72\\ 62\\ 54\\ 28\\ 28\\ 28\\ 28\\ 28\\ 28\\ 28\\ 28\\ 28\\ 28$	$\begin{array}{c} 83\\ 81\\ 80\\ 78\\ 76\\ 75\\ 72\\ 69\\ 66\\ 62\\ 57\\ 53\\ 48\\ 41\\ 30\\ 27\\ 14\\ 15\\ 16\\ 16\\ 17\\ 19\\ 920\\ 20\\ 21\\ 223\\ 23\\ 24\\ 25\\ 27\\ 29\\ 33\\ 84\\ 58\\ 64\\ 65\\ 78\\ 92\\ 40\\ 10\\ 10\\ 10\\ 10\\ 10\\ 10\\ 10\\ 10\\ 10\\ 1$	Pass Pass Pass Pass Pass Pass Pass Pass
0.0377	23	24	104	Pass

Water Quality

Water QualityWater Quality BMP Flow and Volume for POC #1On-line facility volume:0.0112 acre-feetOn-line facility target flow:0.0056 cfs.Adjusted for 15 min:0.0036 cfs.Off-line facility target flow:0.0037 cfs.Adjusted for 15 min:0.0037 cfs.

LID Report

LID Technique	Used for Treatment ?		Volume Through Facility (ac-ft)	Infiltration Volume (ac-ft)	Cumulative Volume Infiltration Credit	Percent Volume Infiltrated	Water Quality	Percent Water Quality Treated	Comment
Vault 1 POC		58.08				0.00			
Total Volume Infiltrated		58.08	0.00	0.00		0.00	0.00	0%	No Treat. Credit
Compliance with LID Standard 8% of 2-yr to 50% of 2-yr									Duration Analysis Result = Passed

Model Default Modifications

Total of 0 changes have been made.

PERLND Changes

No PERLND changes have been made.

IMPLND Changes

No IMPLND changes have been made.

Appendix Predeveloped Schematic

 Basin 1 10.41ac		

Mitigated Schematic



Predeveloped UCI File

RUN

GLOBAL WWHM4 model simulation END 3 0 START 1948 10 01 2009 09 30 RUN INTERP OUTPUT LEVEL RESUME 0 RUN 1 UNIT SYSTEM 1 END GLOBAL FILES <File> <Un#> <-----File Name----->*** * * * <-ID-> 26 default[2].wdm WDM MESSU 25 Predefault[2].MES 27 Predefault[2].L61 28 Predefault[2].L62 30 POCdefault [2] 1.dat END FILES OPN SEQUENCE 12 INGRP INDELT 00:15 PERLND 501 COPY DISPLY 1 END INGRP END OPN SEQUENCE DISPLY DISPLY-INFO1 # - #<-----Title---->***TRAN PIVL DIG1 FIL1 PYR DIG2 FIL2 YRND 1 Basin 1 1 2 30 9 MAX END DISPLY-INFO1 END DISPLY COPY TIMESERIES # - # NPT NMN *** 1 1 501 1 1 1 END TIMESERIES END COPY GENER OPCODE # # OPCD *** END OPCODE PARM K *** # # END PARM END GENER PERLND GEN-INFO <PLS ><-----Name----->NBLKS Unit-systems Printer *** User t-series Engl Metr *** # - # in out *** 1 1 1 27 0 12 C, Forest, Steep 1 END GEN-INFO *** Section PWATER*** ACTIVITY # -# ATMP SNOW PWATSEDPSTPWGPQALMSTLPESTNITRPHOSTRAC***120010000000 END ACTIVITY PRINT-INFO

 # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC

 12
 0
 0
 0
 0
 0
 0
 1
 9

 END PRINT-INFO

PWAT-PARM1 <PLS > PWATER variable monthly parameter value flags *** # - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT *** 12 0 0 0 0 0 0 0 0 0 0 0 0 0 END PWAT-PARM1 PWAT-PARM2

 <PLS >
 PWATER input info: Part 2

 # - # ***FOREST
 LZSN
 INFILT
 LSUR
 SLSUR
 KVARY
 AGWRC

 12
 0
 4.5
 0.08
 400
 0.15
 0.5
 0.996

 END
 DNAT
 DADM2

 END PWAT-PARM2 PWAT-PARM3 PWAT-PARM3<PLS >PWATER input info: Part 3***# - # ***PETMAXPETMININFEXPINFILD1200220 BASETP AGWETP 2 0 0 0 END PWAT-PARM3 PWAT-PARM4 <PLS > PWATER input info: Part 4 ***
 # - #
 CEPSC
 UZSN
 NSUR
 INTFW
 IRC
 LZETP ***

 12
 0.2
 0.3
 0.35
 6
 0.3
 0.7
 END PWAT-PARM4 PWAT-STATE1 <PLS > *** Initial conditions at start of simulation ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 *** # *** CEPS SURS UZS IFWS LZS AGWS GWVS 0 0 0 0 0 2.5 1 0 12 END PWAT-STATE1 END PERLND IMPLND GEN-INFO <PLS ><-----Name----> Unit-systems Printer *** # - # User t-series Engl Metr *** in out *** END GEN-INFO *** Section IWATER*** ACTIVITY # - # ATMP SNOW IWAT SLD IWG IQAL *** END ACTIVITY PRINT-INFO <ILS > ******* Print-flags ******* PIVL PYR # - # ATMP SNOW IWAT SLD IWG IQAL ******** END PRINT-INFO IWAT-PARM1 <PLS > IWATER variable monthly parameter value flags *** # - # CSNO RTOP VRS VNN RTLI *** END IWAT-PARM1 IWAT-PARM2 <PLS > IWATER input info: Part 2 ***
- # *** LSUR SLSUR NSUR RETSC END IWAT-PARM2 IWAT-PARM3 <PLS > IWATER input info: Part 3 * * * # - # ***PETMAX PETMIN END IWAT-PARM3 IWAT-STATE1 <PLS > *** Initial conditions at start of simulation # - # *** RETS SURS END IWAT-STATE1

SCHEMATIC <--Area--> <-Target-> MBLK *** <-factor-> <Name> # Tbl# *** <-Source-> <Name> # Basin 1*** 0.41 COPY 501 12 0.41 COPY 501 13 PERLND 12 PERLND 12 *****Routing***** END SCHEMATIC NETWORK <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> *** <Name> # <Name> # #<-factor->strg <Name> # # <Name> # # *** COPY 501 OUTPUT MEAN 1 1 48.4 DISPLY 1 INPUT TIMSER 1 <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> *** <Name> # <Name> # #<-factor->strg <Name> # # <Name> # # *** END NETWORK RCHRES GEN-INFO RCHRES Name Nexits Unit Systems Printer *** # - #<----- User T-series Engl Metr LKFG * * * in out * * * END GEN-INFO *** Section RCHRES*** ACTIVITY # - # HYFG ADFG CNFG HTFG SDFG GOFG OXFG NUFG PKFG PHFG *** END ACTIVITY PRINT-INFO # - # HYDR ADCA CONS HEAT SED GOL OXRX NUTR PLNK PHCB PIVL PYR ******** END PRINT-INFO HYDR-PARM1 RCHRES Flags for each HYDR Section *** END HYDR-PARM1 HYDR-PARM2 # - # FTABNO LEN DELTH STCOR KS DB50 *** <----><----><----><----> * * * END HYDR-PARM2 HYDR-INIT RCHRES Initial conditions for each HYDR section *** <----> END HYDR-INIT END RCHRES SPEC-ACTIONS END SPEC-ACTIONS FTABLES END FTABLES EXT SOURCES <-Volume-> <Member> SsysSqap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> *** <Name> # <Name> # tem strg<-factor->strg <Name> # # <Name> # # *** WDM 2 PREC ENGL 1 PERLND 1 999 EXTNL PREC WDM 2 PREC ENGL 1 IMPLND 1 999 EXTNL PREC

END IMPLND

WDM	1 EVAP	ENGL	0.76	PERLND 1	999 EXTNL	PETINP
WDM	1 EVAP	ENGL	0.76	IMPLND 1	999 EXTNL	PETINP
END EXT :	SOURCES					
EXT TARG	ETS					
	-					'sys Tgap Amd ***
			5			tem strg strg***
		MEAN 1 1	48.4	WDM 501	FLOW E	NGL REPL
END EXT '	IARGEIS					
MASS-LIN	K					
<volume></volume>	<-Grp>	<-Member->	<mult></mult>	<target></target>	<-Grp>	<-Member->***
<name></name>		<name> # #</name>	<-factor->	<name></name>		<name> # #***</name>
	INK	12		00.511		
PERLND	PWATER		0.083333	COPY	INPUT	MEAN
END MA	SS-LINK	12				
MASS-L	INK	13				
PERLND	PWATER	IFWO	0.083333	COPY	INPUT	MEAN
END MA	SS-LINK	13				

END MASS-LINK

END RUN

Mitigated UCI File

RUN

GLOBAL WWHM4 model simulation
 START
 1948 10 01
 END
 2009 09 30

 RUN INTERP OUTPUT LEVEL
 3
 0
 RESUME 0 RUN 1 UNIT SYSTEM 1 END GLOBAL FILES <File> <Un#> <-----File Name----->*** * * * <-ID-> WDM 26 default[2].wdm MESSU 25 Mitdefault[2].MES 27 Mitdefault[2].L61 28 Mitdefault[2].L62 28 Mitdefault[2].L62 30 POCdefault[2]1.dat END FILES OPN SEQUENCE NGRP IMPLND 4 IMPLND 5 RCHRES 1 COPY 1 INGRP INDELT 00:15 COPY COPY DISPLY 501 1 END INGRP END OPN SEQUENCE DISPLY DISPLY-INFO1

 # #<-----Title---->***TRAN PIVL DIG1 FIL1
 PYR DIG2 FIL2 YRND

 1
 Vault 1
 MAX
 1
 2
 30
 9

 END DISPLY-INFO1 END DISPLY COPY TIMESERIES # - # NPT NMN *** 1 1 1 501 1 1 END TIMESERIES END COPY GENER OPCODE # # OPCD *** END OPCODE PARM K *** # # END PARM END GENER PERLND GEN-INFO <PLS ><----Name---->NBLKS Unit-systems Printer *** # - # User t-series Engl Metr *** * * * in out END GEN-INFO *** Section PWATER*** ACTIVITY # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *** END ACTIVITY PRINT-INFO # - # ATMP SNOW PWAT SED PST PWG POAL MSTL PEST NITR PHOS TRAC ******* END PRINT-INFO

PWAT-PARM1 <PLS > PWATER variable monthly parameter value flags *** # - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT *** END PWAT-PARM1 PWAT-PARM2 <PLS > PWATER input info: Part 2 ***
- # ***FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC END PWAT-PARM2 PWAT-PARM3 WAT-PARM3 <PLS > PWATER input info: Part 3 *** # - # ***PETMAX PETMIN INFEXP INFILD DEEPFR BASETP AGWETP END PWAT-PARM3 PWAT-PARM4 <PLS > PWATER input info: Part 4 ***
- # CEPSC UZSN NSUR INTFW IRC LZETP *** END PWAT-PARM4 PWAT-STATE1 <PLS > *** Initial conditions at start of simulation ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 *** # - # *** CEPS SURS UZS IFWS LZS AGWS GWVS END PWAT-STATE1 END PERLND IMPLND GEN-INFO <PLS ><-----Name----> Unit-systems Printer *** # - # User t-series Engl Metr *** in out *** 4 ROOF TOPS/FLAT 5 DRIVEWAYS/FLAT 1 1 1 27 0 1 1 1 27 0 0 END GEN-INFO *** Section IWATER*** ACTIVITY
 # - # ATMP SNOW IWAT SLD IWG IQAL

 4
 0
 1
 0
 0
 0

 5
 0
 0
 1
 0
 0
 0
 END ACTIVITY PRINT-INFO <ILS > ******* Print-flags ******* PIVL PYR

 # # ATMP SNOW IWAT SLD IWG IQAL

 4
 0
 0
 4
 0
 0
 1
 9

 5
 0
 0
 4
 0
 0
 1
 9

 END PRINT-INFO IWAT-PARM1 <PLS > IWATER variable monthly parameter value flags ***

 # - # CSNO RTOP VRS VNN RTLI

 4
 0
 0
 0

 5
 0
 0
 0
 0

 END IWAT-PARM1 IWAT-PARM2

 <PLS >
 IWATER input info: Part 2

 # - # ***
 LSUR
 SLSUR
 NSUR
 RETSC

 4
 400
 0.01
 0.1
 0.1

 5
 400
 0.01
 0.1
 0.1

 END IWAT-PARM2 IWAT-PARM3 IWATER input info: Part 3 <PLS > * * * # - # ***PETMAX PETMIN 0 0 4 5 0 0

END IWAT-PARM3 IWAT-STATE1 <PLS > *** Initial conditions at start of simulation # - # *** RETS SURS 4 0 0 0 0 5 END IWAT-STATE1 END IMPLND SCHEMATIC <--Area--> <-Target-> MBLK *** <-factor-> <Name> # Tbl# *** <-Source-> <Name> # Basin 1*** 0.11 RCHRES 1 5 0.3 RCHRES 1 5 IMPLND 4 IMPLND 5 *****Routing***** 0.11 COPY 1 15 0.3 COPY 1 15 1 COPY 501 16 IMPLND 4 IMPLND 5 IMPLND RCHRES 1 END SCHEMATIC NETWORK <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> *** <Name> # _ <Name> # #<-factor->strg <Name> # # _ <Name> # # *** COPY 501 OUTPUT MEAN 1 1 48.4 DISPLY 1 INPUT TIMSER 1 <-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> *** <Name> # <Name> # #<-factor->strg <Name> # # <Name> # # *** END NETWORK RCHRES GEN-INFO RCHRES Name Nexits Unit Systems Printer *** # - #<----> User T-series Engl Metr LKFG in out *** *** 1 1 1 1 28 0 1 1 Vault 1 END GEN-INFO *** Section RCHRES*** ACTIVITY
 #
 +
 HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFG PKFG PHFG ***

 1
 1
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 0
 END ACTIVITY PRINT-INFO # - # HYDR ADCA CONS HEAT SED GQL OXRX NUTR PLNK PHCB PIVL PYR ******** 1 4 0 0 0 0 0 0 0 0 0 1 9 END PRINT-INFO HYDR-PARM1 RCHRES Flags for each HYDR Section * * *

 # - # VC A1 A2 A3 ODFVFG for each
 *** ODGTFG for each
 FUNCT for each

 FG FG FG FG FG possible
 exit
 *** possible
 exit

 1
 0
 1
 0
 4
 0
 0
 0
 0
 0
 2
 2
 2
 2

 END HYDR-PARM1 HYDR-PARM2 # - # FTABNO LEN DELTH STCOR KS DB50 * * * <----><----><----><----><----> * * * 1 0.01 0.0 0.0 0.5 0.0 1 END HYDR-PARM2 HYDR-INIT

# - #,	Initial c *** VOL *** ac-ft <>	for eac	l value h possible	of COLIND e exit		*** ue of OUTDGT ible exit ><>
1 END HYDR- END RCHRES	0 -INIT	4.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0 0.0
SPEC-ACTION END SPEC-AC FTABLES						
FTABLE 92 4	1					
Depth	Area	Volume	Outflow1	Velocity		
(ft) 0.000000	(acres) 0.019729	(acre-ft) 0.000000	(cfs) 0.000000	(ft/sec)	(Minutes) ***	
0.088889	0.019729	0.001754	0.001049			
0.177778 0.266667	0.019729 0.019729	0.003507 0.005261	0.001483 0.001816			
0.355556	0.019729	0.007015	0.002097			
0.444444 0.533333	0.019729 0.019729	0.008768 0.010522	0.002345 0.002568			
0.622222 0.711111	0.019729 0.019729	0.012276 0.014029	0.002774 0.002966			
0.800000	0.019729	0.014029	0.002988			
0.888889 0.977778	0.019729 0.019729	0.017537 0.019290	0.003316 0.003478			
1.066667	0.019729	0.021044	0.003632			
1.155556 1.244444	0.019729 0.019729	0.022798 0.024551	0.003781 0.003923			
1.333333 1.422222	0.019729 0.019729	0.026305 0.028059	0.004061 0.004194			
1.511111	0.019729	0.029812	0.004323			
1.600000 1.688889	0.019729 0.019729	0.031566 0.033320	0.004449 0.004571			
1.777778	0.019729	0.035073	0.004689			
1.866667 1.955556	0.019729 0.019729	0.036827 0.038581	0.004805 0.004918			
2.044444 2.133333	0.019729 0.019729	0.040334 0.042088	0.005029 0.005137			
2.222222	0.019729	0.043842	0.005243			
2.311111 2.400000	0.019729 0.019729	0.045595 0.047349	0.005347 0.005448			
2.488889	0.019729	0.049103	0.005548			
2.577778 2.666667	0.019729 0.019729	0.050856 0.052610	0.005647 0.005743			
2.755556 2.844444	0.019729 0.019729	0.054364 0.056117	0.005838 0.005931			
2.933333	0.019729	0.057871	0.006023			
3.022222 3.111111	0.019729 0.019729	0.059625 0.061378	0.006114 0.006203			
3.200000 3.288889	0.019729 0.019729	0.063132 0.064886	0.006291 0.006378			
3.377778	0.019729	0.066639	0.006464			
3.466667 3.555556	0.019729 0.019729	0.068393 0.070147	0.006548			
3.644444	0.019729	0.071900	0.006714			
3.733333 3.822222	0.019729 0.019729	0.073654 0.075408	0.006795 0.006876			
3.911111 4.000000	0.019729 0.019729	0.077161 0.078915	0.006955 0.007034			
4.088889	0.019729	0.080669	0.007112			
4.177778 4.266667	0.019729 0.019729	0.082422 0.084176	0.009501 0.010765			
4.355556 4.444444	0.019729 0.019729	0.085930 0.087683	0.011718 0.012521			
4.533333	0.019729	0.089437	0.013231			
4.622222 4.711111	0.019729 0.019729	0.091191 0.092945	0.013877 0.014475			
4.800000	0.019729	0.094698	0.015034			

4.888889 0.0197 4.977778 0.0197 5.066667 0.0197 5.155556 0.0197 5.244444 0.0197 5.33333 0.0197 5.422222 0.0197 5.51111 0.0197 5.600000 0.0197 5.600000 0.0197 5.688889 0.0197 5.777778 0.0197 5.955556 0.0197 6.044444 0.0197 6.31111 0.0197 6.40000 0.0197 6.48889 0.0197 6.40000 0.0197 6.48889 0.0197 6.577778 0.0197 6.666667 0.0197 6.666667 0.0197 6.755556 0.0197 6.666667 0.0197 6.755556 0.0197 6.844444 0.0197 7.022222 0.0197 7.11111 0.0197 7.200000 0.0197 7.288889 0.0197 7.466667 0.0197 7.466667 0.0197 7.555556 0.0197 7.466667 0.0197 7.555556 0.0197 7.466667 0.0197 7.33333 0.0197 7.33333 0.0197 7.644444 0.0197 7.733333 0.0197 7.822222 0.0197 7.91111 0.0197 8.000000 0.0197 8.088889 0.0197 END FTABLE 1 END FTABLE 1 END FTABLES	29 0.098206 0 29 0.099959 0 29 0.101713 0 29 0.103467 0 29 0.105220 0 29 0.106974 0 29 0.106974 0 29 0.110481 0 29 0.110481 0 29 0.112235 0 29 0.117496 0 29 0.117496 0 29 0.122757 0 29 0.124511 0 29 0.124511 0 29 0.128018 0 29 0.128018 0 29 0.135033 0 29 0.135033 0 29 0.136786 0 29 0.140294 0 29 0.142047 2 29 0.147308 2 29 0.147308 2 29 0.147308 2 29 0.149062 2 <t< th=""><th>0.015562 0.016065 0.016544 0.017005 0.017586 0.018935 0.020518 0.021190 0.021819 0.022415 0.022985 0.023533 0.024061 0.024573 0.025554 0.025554 0.026025 0.026025 0.026025 0.026025 0.026486 0.026936 0.027377 0.027810 0.027810 0.028234 0.02850 0.028450 0.027377 1.434322 2.404972 3.416915 4.357037 5.128737 5.681171 6.046105 6.458414 6.797204 7.119846 7.428451</th><th></th><th></th><th></th><th></th><th></th></t<>	0.015562 0.016065 0.016544 0.017005 0.017586 0.018935 0.020518 0.021190 0.021819 0.022415 0.022985 0.023533 0.024061 0.024573 0.025554 0.025554 0.026025 0.026025 0.026025 0.026025 0.026486 0.026936 0.027377 0.027810 0.027810 0.028234 0.02850 0.028450 0.027377 1.434322 2.404972 3.416915 4.357037 5.128737 5.681171 6.046105 6.458414 6.797204 7.119846 7.428451					
EXT SOURCES <-Volume-> <member <name> # <name> WDM 2 PREC WDM 2 PREC WDM 1 EVAP WDM 1 EVAP</name></name></member 	<pre>> SsysSgap<mt #="" 0.76="" 0.76<="" 1="" engl="" pre="" strg<-fac="" tem=""></mt></pre>	ult>Tran ctor->strg	<-Target <name> PERLND IMPLND PERLND IMPLND</name>	# # 1 999 1 999	EXTNL EXTNL EXTNL	<-Member <name> ‡ PREC PREC PETINP PETINP</name>	
END EXT SOURCES							
RCHRES 1 HYDR RCHRES 1 HYDR	<name> # #<-fac RO 1 1 STAGE 1 1 MEAN 1 1</name>	ctor->strg 1 1	<name> WDM 100 WDM 100 WDM 70</name>		ne> t V El G El V El	sys Tgap Lem strg NGL NGL NGL	
	<-Member-> <mu <name> # #<-fac 5 SURO 0.08 5</name></mu 		<target> <name> RCHRES</name></target>		<-Grp>	<-Member <name> ‡ IVOL</name>	
IMPLND IWATER	15 SURO 0.08 15	83333	COPY		INPUT	MEAN	

MASS-LINK 16 RCHRES ROFLOW END MASS-LINK 16

END MASS-LINK

END RUN

Disclaimer

Legal Notice

This program and accompanying documentation are provided 'as-is' without warranty of any kind. The entire risk regarding the performance and results of this program is assumed by End User. Clear Creek Solutions Inc. and the governmental licensee or sublicensees disclaim all warranties, either expressed or implied, including but not limited to implied warranties of program and accompanying documentation. In no event shall Clear Creek Solutions Inc. be liable for any damages whatsoever (including without limitation to damages for loss of business profits, loss of business information, business interruption, and the like) arising out of the use of, or inability to use this program even if Clear Creek Solutions Inc. or their authorized representatives have been advised of the possibility of such damages. Software Copyright © by : Clear Creek Solutions, Inc. 2005-2022; All Rights Reserved.

Clear Creek Solutions, Inc. 6200 Capitol Blvd. Ste F Olympia, WA. 98501 Toll Free 1(866)943-0304 Local (360)943-0304

www.clearcreeksolutions.com